

Design Information for Metal Plate Connected Truss

Designed By: Missing employee KS

Customer Address:

Manufacturer Address:

Results for: 900 (Frame Design Method - Buckling by Joint Compliance)

НОРМА: CSN 73 00 35

ПРОЛЕТ: 8400 ШАГ: 900

НАГРУЗКА: 2500.000-350.000-0.000-260.000 INCR: LS= 1.00 LBR= 3.00 ГВОЗДЬ= 0.62

ЭЛЕМЕНТ 5-6 ВП МАКС CSI= 0.32 + 0.62 = 0.94

ЭЛЕМЕНТ 2-8 НП МАКС CSI= 0.48 + 0.23 = 0.71

CALC. L. L. DEFLECTION = -11 (L/>780) AT 2-8 INPUT L/200.

МАКС. ВЕРТИК. ДЕФОРМАЦИЯ = -11 (L/>780) V 2-8 INPUT L/200

МАКС. ГОРИЗОНТ. ДЕФОРМАЦИЯ = 3 В УЗЛЕ 6

МАКС. НЕЗАКРЕПЛЕННАЯ ДЛИНА ВП = 0

МАКС. ДЛИНА НП = 0

TOP CHORD: 50x150 SM-JD SI -G

BOTTOM CHORD: 50x150 SM-JD SI -G

WEBS: 50x150 SM-JD SI -G

Design Service Supplied By:

PHP

Voice:

FAX :

21 Май 2012

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Truss Geometry: (Frame Design Method - Buckling by Joint Compliance)

Total Span:	8400	Truss Spacing:	900
TC-Pitch left:	23.00deg	TC-Pitch right:	23.00deg
TC-Purlin Spacing:	350	BC-Purlin Spacing:	0
Overall Height:	2200	Overall Length:	9600
Left Top Overhang:	600	Right Top Overhang:	600

Joint Coordinates and Restraint Conditions:

Jnt Id.	Coordinates - (mm)		Restraint			Inclination Deg	Master Jnt No.
	X	Y	X	Y	Z		
2	0	75	Fix	Fix		0.00	
9	5500	75					
1	-600	-173					
4	4200	1864					
7	9000	-173					
8	4200	75					
3	1997	929					
5	6403	929					
6	8400	75	Fix			0.00	
10	1400	674					
11	1800	845					
12	2200	1015					
13	6200	1015					
14	6600	845					
15	7000	674					

Member Information

Panel	Design	Type	Loading	Purlin Spacing (mm)	Lengths Actual (mm)	Buckling (mm)	Moment Midpnl (mm)	Lengths Pnlpnt (mm)
Bottom Chord	at	0.000 deg. (0.00deg)						
2-8 Heel		Porch			4200	1988	100	100
6-8 Heel		Porch			4200	2520	100	100
Top Chord	at	23.000 deg. (23.00deg)						
1-2 Extension		Outside Ext			649	1299	100	100
2-3 Heel		Regular			2172	841	100	100
3-4 Regular		Regular			2393	1436	100	100
Top Chord	at	-23.000 deg. (-23.00deg)						
4-5 Regular		Regular			2393	841	100	100
5-6 Heel		Regular			2172	898	100	100
6-7 Extension		Outside Ext			649	1299	100	100
Web	at	-20.109 deg. (-20.11deg)						
3-8 Regular		Regular			2363	2363	100	100
Web	at	90.000 deg. (90.00deg)						
4-8 Regular		Regular			1789	1789	100	100
Web	at	20.109 deg. (20.11deg)						
5-8 Regular		Regular			2363	2363	100	100

Mbr	Type	End Cond.	Length	Area	Section Modulus	Moment of Inertia X	Y
		Start End	(mm)	(mm2)	(m3)	(mm4)	(mm4)
Bottom Chord							
2-8	Beam	Pinned Rigid	4200	7500.0	0.000	1.41e+007	1.56e+006
8-9	Beam	Rigid Rigid	1300	7500.0	0.000	1.41e+007	1.56e+006
6-9	Beam	Rigid Pinned	2900	7500.0	0.000	1.41e+007	1.56e+006
Top Chord							
1-2	Beam	Pinned Rigid	649	7500.0	0.000	1.41e+007	1.56e+006
2-10	Beam	Rigid Rigid	1523	7500.0	0.000	1.41e+007	1.56e+006
10-11	Beam	Rigid Rigid	435	7500.0	0.000	1.41e+007	1.56e+006
3-11	Beam	Rigid Rigid	214	7500.0	0.000	1.41e+007	1.56e+006

Mbr	Type	End Start	Cond. End	Length (mm)	Area (mm2)	Section Modulus (m3)	Moment of X (mm4)	Inertia Y (mm4)
3-12	Beam	Rigid	Rigid	221	7500.0	0.000	1.41e+007	1.56e+006
4-12	Beam	Rigid	Pinned	2173	7500.0	0.000	1.41e+007	1.56e+006
Top Chord								
4-13	Beam	Pinned	Rigid	2173	7500.0	0.000	1.41e+007	1.56e+006
5-13	Beam	Rigid	Rigid	221	7500.0	0.000	1.41e+007	1.56e+006
5-14	Beam	Rigid	Rigid	214	7500.0	0.000	1.41e+007	1.56e+006
14-15	Beam	Rigid	Rigid	435	7500.0	0.000	1.41e+007	1.56e+006
6-15	Beam	Rigid	Rigid	1523	7500.0	0.000	1.41e+007	1.56e+006
6-7	Beam	Rigid	Pinned	649	7500.0	0.000	1.41e+007	1.56e+006
Web								
3-8	Beam	Pinned	Pinned	2363	7500.0	0.000	1.41e+007	1.56e+006
Web								
4-8	Beam	Pinned	Pinned	1789	7500.0	0.000	1.41e+007	1.56e+006
Web								
5-8	Beam	Pinned	Pinned	2363	7500.0	0.000	1.41e+007	1.56e+006

Basic Allowable Stresses (MPa)

Mbr	Fb	Ft	Fv	Fcp	Fc	E
2-8	22.000	13.000	3.600	5.100	20.000	6700.000
8-9	22.000	13.000	3.600	5.100	20.000	6700.000
6-9	22.000	13.000	3.600	5.100	20.000	6700.000
1-2	22.000	13.000	3.600	5.100	20.000	6700.000
2-10	22.000	13.000	3.600	5.100	20.000	6700.000
10-11	22.000	13.000	3.600	5.100	20.000	6700.000
3-11	22.000	13.000	3.600	5.100	20.000	6700.000
3-12	22.000	13.000	3.600	5.100	20.000	6700.000
4-12	22.000	13.000	3.600	5.100	20.000	6700.000
4-13	22.000	13.000	3.600	5.100	20.000	6700.000
5-13	22.000	13.000	3.600	5.100	20.000	6700.000
5-14	22.000	13.000	3.600	5.100	20.000	6700.000
14-15	22.000	13.000	3.600	5.100	20.000	6700.000
6-15	22.000	13.000	3.600	5.100	20.000	6700.000
6-7	22.000	13.000	3.600	5.100	20.000	6700.000
3-8	22.000	13.000	3.600	5.100	20.000	6700.000
4-8	22.000	13.000	3.600	5.100	20.000	6700.000
5-8	22.000	13.000	3.600	5.100	20.000	6700.000

Loading

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LC 1: $1.2\mu\text{H} + 1.4\text{CH}\mu\text{1}/\mu\text{2} + 1.25\text{BH}$

Пояс	№.	Тип	Распред.	Нагр. - Напр.	Напр.Проек.	Нач.	Стоим1	Конец	Стоим2	Панель(и)
Верхний		Постоянная	Соединенное	Вниз	X	Нет	-600	0.065	9000	Верхний пояс- Постоянный & В
Нижний		Постоянная	Соединенное	Вниз	X	Нет	0	0.059	8400	Нижний пояс-Постоянный & Ниж

$$LC \quad 2: 1.2\pi H + 1.4C_H \mu_2 / \mu_1 + 1.25B_H$$

Пояс	№.	Тип	Распред.	Нагр. - Напр.	Напр.Проек.	Нач.	Стоим1	Конец	Стоим2	Панель(и)
Верхний		Постоянная	Соединенное	Вниз	X	Нет	-600	0.065	9000	Верхний пояс- Постоянный & В
Нижний		Постоянная	Соединенное	Вниз	X	Нет	0	0.059	8400	Нижний пояс-Постоянный & Ниж

LC 3: $1.2\mu_1 + 1.4\sigma_1/\mu_2 + 1.25V_{spr} + 1.25V_N$

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Пояс	№.	Тип	Распред.	Нагр. - Напр.	Напр.Проек.	Нач.	Стоим1	Конец	Стоим2	Панель(и)
Верхний		Постоянная	Соединенное	Вниз	X	Нет	-600	0.065	9000	Верхний пояс- Постоянный & В
Нижний		Постоянная	Соединенное	Вниз	X	Нет	0	0.059	8400	Нижний пояс-Постоянный & Ниж

LC 4: $1.2\pi n + 1.4\sigma n^2/\mu_1 + 1.25V_{сл} + 1.25V_H$

Пояс	№.	Тип	Распред.	Нагр. - Напр.	Напр.Проек.	Нач.	Стоим1	Конец	Стоим2	Панель(и)
Верхний		Постоянная	Соединенное	Вниз	X	Нет	-600	0.065	9000	Верхний пояс- Постоянный & В
Нижний		Постоянная	Соединенное	Вниз	X	Нет	0	0.059	8400	Нижний пояс-Постоянный & Ниж

LC 5: 1.0ПН+1.4Вспр

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Пояс	№.	Тип	Распред.	Нагр. - Напр.	Напр.Проек.	Нач.	Стоим1	Конец	Стоим2	Панель(и)
Верхний		Постоянная	Соединенное	Вниз	X	Нет	-600	0.065	9000	Верхний пояс- Постоянный & В
Нижний		Постоянная	Соединенное	Вниз	X	Нет	0	0.059	8400	Нижний пояс-Постоянный & Ниж

LC 6: 1.0ПН+1.4Всл

Пояс	№	Тип	Распред.	Нагр.-Напр.	Напр.Проек.	Нач.	Стоим1	Конец	Стоим2	Панель(и)
Верхний		Постоянная	Соединенное	Вниз	X	Нет	-600	0.065	9000	Верхний пояс- Постоянный & В
Нижний		Постоянная	Соединенное	Вниз	X	Нет	0	0.059	8400	Нижний пояс-Постоянный & Ниж

LC 7: $1.2\text{пн} + 1.4 * 0.5\mu\text{1CH_п} + 1.25\text{Вн}$

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Пояс	№	Тип	Распред.	Нагр.-Напр.	Напр.Проек.	Нач.	Стоим1	Конец	Стоим2	Панель(и)
Верхний		Постоянная	Соединенное	Вниз	X	Нет	-600	0.065	9000	Верхний пояс- Постоянный & В
Нижний		Постоянная	Соединенное	Вниз	X	Нет	0	0.059	8400	Нижний пояс-Постоянный & Ниж

$$LC \quad 8: 1.2\pi_H + 1.4 * 0.5\mu_{1CH_L} + 1.25\pi_H$$

Пояс	№	Тип	Распред.	Нагр.-Напр.	Напр.Проек.	Нач.	Стоим1	Конец	Стоим2	Панель(и)
Верхний		Постоянная	Соединенное	Вниз	X	Нет	-600	0.065	9000	Верхний пояс- Постоянный & В
Нижний		Постоянная	Соединенное	Вниз	X	Нет	0	0.059	8400	Нижний пояс-Постоянный & Ниж

LC 9: $1.2\text{пн} + 1.4 \cdot 0.5\mu\text{сн_п} + 1.25\text{Вспр} + 1.25\text{Вн}$

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Пояс	№	Тип	Распред.	Нагр.-Напр.	Напр.Проек.	Нач.	Стоим1	Конец	Стоим2	Панель(и)
Верхний		Постоянная	Соединенное	Вниз	X	Нет	-600	0.065	9000	Верхний пояс- Постоянный & В
Нижний		Постоянная	Соединенное	Вниз	X	Нет	0	0.059	8400	Нижний пояс-Постоянный & Ниж

LC 10: $1.2\text{ПН} + 1.4 \cdot 0.5\mu 1\text{CH}_{\text{Л}} + 1.4\text{ВсЛ} + 1.25\text{ВН}$

Пояс	№.	Тип	Распред.	Нагр.-Напр.	Напр.Проек.	Нач.	Стоим1	Конец	Стоим2	Панель(и)
Верхний		Постоянная	Соединенное	Вниз	X	Нет	-600	0.065	9000	Верхний пояс- Постоянный & В
Нижний		Постоянная	Соединенное	Вниз	X	Нет	0	0.059	8400	Нижний пояс-Постоянный & Ниж

LC 13: 1.0ПН+1.4W ьтнт F-G

Пояс	№.	Тип	Распред.	Нагр.-Напр.	Напр.	Проек.	Нач.	Стоим1	Конец	Стоим2	Панель(и)
Верхний		Постоянная	Соединенное	Вниз	X	Нет	-600	0.065	9000		Верхний пояс- Постоянный & В
Нижний		Постоянная	Соединенное	Вниз	X	Нет	0	0.059	8400		Нижний пояс-Постоянный & Ниж

LC 14: 1.0DL+1.4W ьтнт H

Пояс	№.	Тип	Распред.	Нагр.-Напр.	Напр.	Проек.	Нач.	Стоим1	Конец	Стоим2	Панель(и)
Верхний		Постоянная	Соединенное	Вниз	X	Нет	-600	0.065	9000		Верхний пояс- Постоянный & В
Нижний		Постоянная	Соединенное	Вниз	X	Нет	0	0.059	8400		Нижний пояс-Постоянный & Ниж

LC 20: 1.2ПН+1.4МВ

Пояс	№.	Тип	Распред.	Нагр.-Напр.	Напр.	Проек.	Нач.	Стоим1	Конец	Стоим2	Панель(и)
Верхний		Постоянная	Соединенное	Вниз	X	Нет	-600	0.065	9000		Верхний пояс- Постоянный & В
Нижний		Постоянная	Соединенное	Вниз	X	Нет	0	0.059	8400		Нижний пояс-Постоянный & Ниж

LC 21: Def: ПН+ВН+СНμ1/μ2

Пояс	№.	Тип	Распред.	Нагр.-Напр.	Напр.	Проек.	Нач.	Стоим1	Конец	Стоим2	Панель(и)
Верхний		Постоянная	Соединенное	Вниз	X	Нет	-600	0.065	9000		Верхний пояс- Постоянный & В
Нижний		Постоянная	Соединенное	Вниз	X	Нет	0	0.059	8400		Нижний пояс-Постоянный & Ниж

LC 22: Def: ПН+ВН+СНμ2/μ1

Пояс	№.	Тип	Распред.	Нагр.-Напр.	Напр.	Проек.	Нач.	Стоим1	Конец	Стоим2	Панель(и)
Верхний		Постоянная	Соединенное	Вниз	X	Нет	-600	0.065	9000		Верхний пояс- Постоянный & В
Нижний		Постоянная	Соединенное	Вниз	X	Нет	0	0.059	8400		Нижний пояс-Постоянный & Ниж

LC 23: Def: ПН+ВН+СНμ1/μ2+Вспр

Пояс	№.	Тип	Распред.	Нагр.-Напр.	Напр.	Проек.	Нач.	Стоим1	Конец	Стоим2	Панель(и)
Верхний		Постоянная	Соединенное	Вниз	X	Нет	-600	0.065	9000		Верхний пояс- Постоянный & В
Нижний		Постоянная	Соединенное	Вниз	X	Нет	0	0.059	8400		Нижний пояс-Постоянный & Ниж

LC 25: Def: ПН+ВН+0.5μ1СН_П

Пояс	№.	Тип	Распред.	Нагр.-Напр.	Напр.	Проек.	Нач.	Стоим1	Конец	Стоим2	Панель(и)
Верхний		Постоянная	Соединенное	Вниз	X	Нет	-600	0.065	9000		Верхний пояс- Постоянный & В
Нижний		Постоянная	Соединенное	Вниз	X	Нет	0	0.059	8400		Нижний пояс-Постоянный & Ниж

LC 26: Def: ПН+ВН+0.5μ1СН_Л

Пояс	№.	Тип	Распред.	Нагр.-Напр.	Напр.	Проек.	Нач.	Стоим1	Конец	Стоим2	Панель(и)
Верхний		Постоянная	Соединенное	Вниз	X	Нет	-600	0.065	9000		Верхний пояс- Постоянный & В
Нижний		Постоянная	Соединенное	Вниз	X	Нет	0	0.059	8400		Нижний пояс-Постоянный & Ниж

LC 27: Def: ПН+ВН+0.5μ1СН_П+Вспр

Пояс	№.	Тип	Распред.	Нагр.-Напр.	Напр.	Проек.	Нач.	Стоим1	Конец	Стоим2	Панель(и)
Верхний		Постоянная	Соединенное	Вниз	X	Нет	-600	0.065	9000		Верхний пояс- Постоянный & В
Нижний		Постоянная	Соединенное	Вниз	X	Нет	0	0.059	8400		Нижний пояс-Постоянный & Ниж

LC 28: Def: ПН+ВН+0.5μ1СН_Л +Всл

Пояс	№.	Тип	Распред.	Нагр.-Напр.	Напр.	Проек.	Нач.	Стоим1	Конец	Стоим2	Панель(и)
Верхний		Постоянная	Соединенное	Вниз	X	Нет	-600	0.065	9000		Верхний пояс- Постоянный & В
Нижний		Постоянная	Соединенное	Вниз	X	Нет	0	0.059	8400		Нижний пояс-Постоянный & Ниж

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LOADCASE 1: Lumber DOL = 3.00 1.2ПН+1.4СНμ1/μ2+1.25ВН

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Truss Spacing: 900

Loading (kN/m) (Global Reference Frame)

Panel		Start		End		
		Vert./ Normal	Horiz./ Parallel	Vert./ Normal	Horiz./ Parallel	
Верх. пояс						
T1	1-2	3.014	0.000	3.014	0.000	proj.
T1	2-10	3.017	0.000	3.017	0.000	proj.
T1	10-11	3.017	0.000	3.017	0.000	proj.
T1	11-3	3.017	0.000	3.017	0.000	proj.
T1	3-12	3.016	0.000	3.016	0.000	proj.
T1	12-4	3.016	0.000	3.016	0.000	proj.
T1	4-13	3.520	0.000	3.520	0.000	proj.
T1	13-5	3.520	0.000	3.520	0.000	proj.
T1	5-14	3.521	0.000	3.521	0.000	proj.
T1	14-15	3.521	0.000	3.521	0.000	proj.
T1	15-6	3.521	0.000	3.521	0.000	proj.
T1	6-7	3.518	0.000	3.518	0.000	proj.
Нижний пояс						
B1	2-8	0.352	0.000	0.352	0.000	proj.
B1	8-9	0.352	0.000	0.352	0.000	proj.
B2	9-6	0.352	0.000	0.352	0.000	proj.

Диагонали

Note: Loads not marked projected, (proj), are loads normal to and parallel to the member.

Member Forces (Local Reference Frame)

		Panel Start			Panel End			Max.
		Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Panel Moment
Верх. пояс								
T1	1-2	0.000	0.000	0.000	1.671	0.691	-0.542	-0.136
T1	2-3	2.445	-30.056	0.542	3.093	-27.687	-1.247	-0.930
T1	3-4	3.579	-20.910	1.247	2.537	-18.314	0.000	1.260
T1	4-5	2.949	-18.140	0.000	4.190	-21.169	-1.485	1.458
T1	5-6	3.624	-29.115	1.485	2.840	-31.879	-0.633	-1.114
T1	6-7	1.950	0.807	0.633	0.000	0.000	0.000	-0.158
Нижний пояс								
B1	2-8	0.598	26.673	0.000	0.881	26.673	-0.595	0.508
B1	8-9	0.881	28.194	0.595	-0.423	28.194	0.253	-0.097
B2	9-6	0.423	28.194	-0.253	0.598	28.194	0.000	0.508
Диагонали								
W1	3-8	0.000	-9.463	0.000	0.000	-9.463	0.000	0.000
W2	8-4	0.000	9.194	0.000	0.000	9.194	0.000	0.000
W1	8-5	0.000	-11.095	0.000	0.000	-11.095	0.000	0.000

Reactions (Global Reference Frame)

Joint Id.	Vertical (kN)	Horiz. (kN)	Moment (kNm)	Bearing Width (mm)
2	-16.474	0.000	0.000	
6	-17.857	0.000	0.000	

Stress Report for Loadcase 1.

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Member Design Forces (0 Axial & Moment: NO stress check needed)

Panel		Start		Midpanel		End	
		Axial	Moment	Axial	Moment	Axial	Moment
Верх. пояс							
T1	1-2	0.000	0.000	0.346	-0.136	0.691	-0.542
T1	2-3	-30.056	-0.643	-29.010	-0.930	-28.395	-1.247
T1	3-4	-20.910	-1.247	-20.791	1.260	-20.671	-0.519
T1	4-5	-20.890	-0.633	-21.030	1.458	-21.169	-1.485
T1	5-6	-29.941	-1.485	-30.665	-1.114	-31.879	-0.777
T1	6-7	0.807	-0.633	0.403	-0.158	0.000	0.000
Нижний пояс							
B1	2-8	26.673	0.000	26.673	0.508	26.673	-0.595
B1	8-9	28.194	-0.595	28.194	-0.097	28.194	0.253
B2	9-6	28.194	0.253	28.194	0.508	28.194	0.000
Диагонали							
W1	3-8	-9.463	0.000	-9.463	0.000	-9.463	0.000
W2	8-4	9.194	0.000	9.194	0.000	9.194	0.000
W1	8-5	-11.095	0.000	-11.095	0.000	-11.095	0.000

Stress Report for Loadcase 1.

Panel	Lumber		P/AF	M/ZF	CSI	SSI	Purlin Space	Ke
Верх. пояс								
T1	1-2	50x150 SI-G	0.01	0.21	0.22	0.15	0	2.00
T1	2-3	50x150 SI-G	0.30	0.49	0.79	0.28	0	0.39
T1	3-4	50x150 SI-G	0.25	0.49	0.74	0.32	0	0.60
T1	4-5	50x150 SI-G	0.25	0.57	0.82	0.37	0	0.35
T1	5-6	50x150 SI-G	0.32	0.58	0.90	0.32	0	0.41
T1	6-7	50x150 SI-G	0.01	0.25	0.26	0.17	0	2.00
Нижний пояс								
B1	2-8	50x150 SI-G	0.44	0.23	0.67	0.08	0	0.47
B1	8-9	50x150 SI-G	0.47	0.23	0.70	0.08	0	0.60
B2	9-6	50x150 SI-G	0.47	0.20	0.66	0.05	0	0.60
Диагонали								
W1	3-8	50x150 SI-G	0.14	0.00	0.14	0.00	0	1.00
W2	8-4	50x150 SI-G	0.15	0.00	0.15	0.00	0	1.00
W1	8-5	50x150 SI-G	0.16	0.00	0.16	0.00	0	1.00

Jnt Id.	Lumber	P/AF	M/ZF	CSI	SSI	Purlin Space	Ke
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LOADCASE 2: Lumber DOL = 3.00 1.2ПН+1.4СНμ2/μ1+1.25ВН

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Truss Spacing: 900

Loading (kN/m) (Global Reference Frame)

Panel		Start		End		
		Vert./ Normal	Horiz./ Parallel	Vert./ Normal	Horiz./ Parallel	
Верх. пояс						
T1	1-2	3.518	0.000	3.518	0.000	proj.
T1	2-10	3.521	0.000	3.521	0.000	proj.
T1	10-11	3.521	0.000	3.521	0.000	proj.
T1	11-3	3.521	0.000	3.521	0.000	proj.
T1	3-12	3.520	0.000	3.520	0.000	proj.
T1	12-4	3.520	0.000	3.520	0.000	proj.
T1	4-13	3.016	0.000	3.016	0.000	proj.
T1	13-5	3.016	0.000	3.016	0.000	proj.
T1	5-14	3.017	0.000	3.017	0.000	proj.
T1	14-15	3.017	0.000	3.017	0.000	proj.
T1	15-6	3.017	0.000	3.017	0.000	proj.
T1	6-7	3.014	0.000	3.014	0.000	proj.
Нижний пояс						
B1	2-8	0.352	0.000	0.352	0.000	proj.
B1	8-9	0.352	0.000	0.352	0.000	proj.
B2	9-6	0.352	0.000	0.352	0.000	proj.

Диагонали

Note: Loads not marked projected, (proj), are loads normal to and parallel to the member.

Member Forces (Local Reference Frame)

		Panel Start			Panel End			Max.
		Shear	Axial	Moment	Shear	Axial	Moment	Panel
		Force	Force		Force	Force		Moment
		(kN)	(kN)	(kNm)	(kN)	(kN)	(kNm)	(kNm)
Верх. пояс								
T1	1-2	0.000	0.000	0.000	1.950	0.807	-0.633	-0.158
T1	2-3	2.840	-31.879	0.633	3.624	-29.115	-1.485	-1.114
T1	3-4	4.190	-21.169	1.485	2.949	-18.140	0.000	1.458
T1	4-5	2.537	-18.314	0.000	3.579	-20.910	-1.247	1.260
T1	5-6	3.094	-27.687	1.247	2.445	-30.056	-0.542	-0.930
T1	6-7	1.671	0.691	0.542	0.000	0.000	0.000	-0.136
Нижний пояс								
B1	2-8	0.598	28.194	0.000	0.881	28.194	-0.595	0.508
B1	8-9	0.881	26.673	0.595	-0.423	26.673	0.253	-0.097
B2	9-6	0.423	26.673	-0.253	0.598	26.673	0.000	0.508
Диагонали								
W1	3-8	0.000	-11.095	0.000	0.000	-11.095	0.000	0.000
W2	8-4	0.000	9.194	0.000	0.000	9.194	0.000	0.000
W1	8-5	0.000	-9.463	0.000	0.000	-9.463	0.000	0.000

Reactions (Global Reference Frame)

Joint Id.	Vertical (kN)	Horiz. (kN)	Moment (kNm)	Bearing Width (mm)
2	-17.857	0.000	0.000	
6	-16.474	0.000	0.000	

Stress Report for Loadcase 2.

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Member Design Forces (0 Axial & Moment: NO stress check needed)

Panel		Start		Midpanel		End	
		Axial	Moment	Axial	Moment	Axial	Moment
Верх. пояс							
T1	1-2	0.000	0.000	0.403	-0.158	0.807	-0.633
T1	2-3	-31.879	-0.777	-30.665	-1.114	-29.941	-1.485
T1	3-4	-21.169	-1.485	-21.030	1.458	-20.890	-0.633
T1	4-5	-20.671	-0.519	-20.791	1.260	-20.910	-1.247
T1	5-6	-28.395	-1.247	-29.010	-0.930	-30.056	-0.643
T1	6-7	0.691	-0.542	0.346	-0.136	0.000	0.000
Нижний пояс							
B1	2-8	28.194	0.000	28.194	0.508	28.194	-0.595
B1	8-9	26.673	-0.595	26.673	-0.097	26.673	0.253
B2	9-6	26.673	0.253	26.673	0.508	26.673	0.000
Диагонали							
W1	3-8	-11.095	0.000	-11.095	0.000	-11.095	0.000
W2	8-4	9.194	0.000	9.194	0.000	9.194	0.000
W1	8-5	-9.463	0.000	-9.463	0.000	-9.463	0.000

Stress Report for Loadcase 2.

Panel	Lumber			P/AF	M/ZF	CSI	SSI	Purlin Space	Ke
Верх. пояс									
T1	1-2	50x150	SI-G	0.01	0.25	0.26	0.17	0	2.00
T1	2-3	50x150	SI-G	0.32	0.58	0.90	0.32	0	0.39
T1	3-4	50x150	SI-G	0.25	0.57	0.82	0.37	0	0.60
T1	4-5	50x150	SI-G	0.25	0.49	0.74	0.32	0	0.35
T1	5-6	50x150	SI-G	0.30	0.49	0.79	0.28	0	0.41
T1	6-7	50x150	SI-G	0.01	0.21	0.22	0.15	0	2.00
Нижний пояс									
B1	2-8	50x150	SI-G	0.47	0.23	0.70	0.08	0	0.47
B1	8-9	50x150	SI-G	0.44	0.23	0.67	0.08	0	0.60
B2	9-6	50x150	SI-G	0.44	0.20	0.64	0.05	0	0.60
Диагонали									
W1	3-8	50x150	SI-G	0.16	0.00	0.16	0.00	0	1.00
W2	8-4	50x150	SI-G	0.15	0.00	0.15	0.00	0	1.00
W1	8-5	50x150	SI-G	0.14	0.00	0.14	0.00	0	1.00

Jnt Id.	Lumber	P/AF	M/ZF	CSI	SSI	Purlin Space	Ke
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LOADCASE 3: Lumber DOL = 3.00 1.2ПН+1.4СНμ1/μ2+1.25Вспр+1.25ВН

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Truss Spacing: 900

Loading (kN/m) (Global Reference Frame)

Panel		Start		End		
		Vert./ Normal	Horiz./ Parallel	Vert./ Normal	Horiz./ Parallel	

Верх. пояс						
T1	1-2	2.971	-0.018	2.971	-0.018	proj.
T1	2-10	2.845	-0.073	2.845	-0.073	proj.
T1	10-11	2.845	-0.073	2.845	-0.073	proj.
T1	11-3	2.845	-0.073	2.845	-0.073	proj.
T1	3-12	2.845	-0.073	2.845	-0.073	proj.
T1	12-4	2.702	-0.133	2.702	-0.133	proj.
T1	4-13	3.651	-0.056	3.651	-0.056	proj.
T1	13-5	3.651	-0.056	3.651	-0.056	proj.
T1	5-14	3.652	-0.056	3.652	-0.056	proj.
T1	14-15	3.652	-0.056	3.652	-0.056	proj.
T1	15-6	3.720	-0.085	3.720	-0.085	proj.
T1	6-7	3.376	0.059	3.376	0.059	proj.
Нижний пояс						
B1	2-8	0.352	0.000	0.352	0.000	proj.
B1	8-9	0.352	0.000	0.352	0.000	proj.
B2	9-6	0.352	0.000	0.352	0.000	proj.

Диагонали

Note: Loads not marked projected, (proj), are loads normal to and parallel to the member.

Member Forces (Local Reference Frame)

		Panel Start			Panel End			Max.
		Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Panel Moment
Верх. пояс								
T1	1-2	0.000	0.000	0.000	1.643	0.691	-0.533	-0.133
T1	2-3	2.322	-29.103	0.533	2.845	-26.734	-1.100	-0.809
T1	3-4	3.173	-20.628	1.100	2.225	-18.032	0.000	1.104
T1	4-5	3.061	-17.677	0.000	4.391	-20.707	-1.591	1.505
T1	5-6	3.865	-29.106	1.591	2.987	-31.871	-0.603	-1.194
T1	6-7	1.858	0.807	0.603	0.000	0.000	0.000	-0.151
Нижний пояс								
B1	2-8	0.597	25.178	0.000	0.882	25.178	-0.599	0.506
B1	8-9	0.882	28.164	0.599	-0.424	28.164	0.250	-0.100
B2	9-6	0.424	28.164	-0.250	0.597	28.164	0.000	0.506
Диагонали								
W1	3-8	0.000	-8.527	0.000	0.000	-8.527	0.000	0.000
W2	8-4	0.000	9.087	0.000	0.000	9.087	0.000	0.000
W1	8-5	0.000	-11.729	0.000	0.000	-11.729	0.000	0.000

Reactions (Global Reference Frame)

Joint Id.	Vertical (kN)	Horiz. (kN)	Moment (kNm)	Bearing Width (mm)
2	-15.960	-0.678	0.000	
6	-17.902	0.000	0.000	

Stress Report for Loadcase 3.

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Member Design Forces (0 Axial & Moment: NO stress check needed)

Panel		Start		Midpanel		End	
		Axial	Moment	Axial	Moment	Axial	Moment
Верх. пояс							
T1	1-2	0.000	0.000	0.346	-0.133	0.691	-0.533
T1	2-3	-29.103	-0.546	-28.038	-0.809	-27.442	-1.100
T1	3-4	-20.628	-1.100	-20.508	1.104	-20.389	-0.459
T1	4-5	-20.427	-0.698	-20.567	1.505	-20.707	-1.591
T1	5-6	-29.933	-1.591	-30.674	-1.194	-31.871	-0.834
T1	6-7	0.807	-0.603	0.403	-0.151	0.000	0.000
Нижний пояс							
B1	2-8	25.178	0.000	25.178	0.506	25.178	-0.599
B1	8-9	28.164	-0.599	28.164	-0.100	28.164	0.250
B2	9-6	28.164	0.250	28.164	0.506	28.164	0.000
Диагонали							
W1	3-8	-8.527	0.000	-8.527	0.000	-8.527	0.000
W2	8-4	9.087	0.000	9.087	0.000	9.087	0.000
W1	8-5	-11.729	0.000	-11.729	0.000	-11.729	0.000

Stress Report for Loadcase 3.

Panel	Lumber		P/AF	M/ZF	CSI	SSI	Purlin Space	Ke
Верх. пояс								
T1	1-2	50x150 SI-G	0.01	0.21	0.22	0.15	0	2.00
T1	2-3	50x150 SI-G	0.29	0.43	0.72	0.25	0	0.39
T1	3-4	50x150 SI-G	0.24	0.43	0.67	0.28	0	0.60
T1	4-5	50x150 SI-G	0.23	0.62	0.85	0.39	0	0.35
T1	5-6	50x150 SI-G	0.32	0.62	0.94	0.35	0	0.41
T1	6-7	50x150 SI-G	0.01	0.24	0.25	0.17	0	2.00
Нижний пояс								
B1	2-8	50x150 SI-G	0.42	0.23	0.65	0.08	0	0.47
B1	8-9	50x150 SI-G	0.47	0.23	0.70	0.08	0	0.60
B2	9-6	50x150 SI-G	0.47	0.20	0.66	0.05	0	0.60
Диагонали								
W1	3-8	50x150 SI-G	0.13	0.00	0.13	0.00	0	1.00
W2	8-4	50x150 SI-G	0.15	0.00	0.15	0.00	0	1.00
W1	8-5	50x150 SI-G	0.17	0.00	0.17	0.00	0	1.00

Jnt Id.	Lumber	P/AF	M/ZF	CSI	SSI	Purlin Space	Ke
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LOADCASE 4: Lumber DOL = 3.00 1.2ПН+1.4СНμ2/μ1+1.25ВсЛ+1.25ВН

Truss Spacing: 900

Loading (kN/m) (Global Reference Frame)

		Start		End		
Panel		Vert./ Normal	Horiz./ Parallel	Vert./ Normal	Horiz./ Parallel	

Верх. пояс						
T1	1-2	3.376	-0.059	3.376	-0.059	proj.
T1	2-10	3.720	0.085	3.720	0.085	proj.
T1	10-11	3.652	0.056	3.652	0.056	proj.
T1	11-3	3.652	0.056	3.652	0.056	proj.
T1	3-12	3.651	0.056	3.651	0.056	proj.
T1	12-4	3.651	0.056	3.651	0.056	proj.
T1	4-13	2.702	0.133	2.702	0.133	proj.
T1	13-5	2.845	0.073	2.845	0.073	proj.
T1	5-14	2.845	0.073	2.845	0.073	proj.
T1	14-15	2.845	0.073	2.845	0.073	proj.
T1	15-6	2.845	0.073	2.845	0.073	proj.
T1	6-7	2.971	0.018	2.971	0.018	proj.
Нижний пояс						
B1	2-8	0.352	0.000	0.352	0.000	proj.
B1	8-9	0.352	0.000	0.352	0.000	proj.
B2	9-6	0.352	0.000	0.352	0.000	proj.

Диагонали

Note: Loads not marked projected, (proj), are loads normal to and parallel to the member.

Member Forces (Local Reference Frame)

		Panel Start			Panel End			Max.
		Shear	Axial	Moment	Shear	Axial	Moment	Panel
		Force	Force		Force	Force		Moment
		(kN)	(kN)	(kNm)	(kN)	(kN)	(kNm)	(kNm)
Верх. пояс								
T1	1-2	0.000	0.000	0.000	1.858	0.807	-0.603	-0.151
T1	2-3	2.989	-31.866	0.603	3.864	-29.101	-1.587	-1.191
T1	3-4	4.389	-20.705	1.587	3.063	-17.675	0.000	1.507
T1	4-5	2.226	-18.030	0.000	3.171	-20.626	-1.096	1.105
T1	5-6	2.843	-26.729	1.096	2.324	-29.098	-0.533	-0.805
T1	6-7	1.643	0.691	0.533	0.000	0.000	0.000	-0.133
Нижний пояс								
B1	2-8	0.598	28.836	0.000	0.882	28.836	-0.597	0.507
B1	8-9	0.882	25.850	0.597	-0.424	25.850	0.252	-0.098
B2	9-6	0.424	25.850	-0.252	0.598	25.850	0.000	0.507
Диагонали								
W1	3-8	0.000	-11.724	0.000	0.000	-11.724	0.000	0.000
W2	8-4	0.000	9.082	0.000	0.000	9.082	0.000	0.000
W1	8-5	0.000	-8.522	0.000	0.000	-8.522	0.000	0.000

Reactions (Global Reference Frame)

Joint Id.	Vertical (kN)	Horiz. (kN)	Moment (kNm)	Bearing Width (mm)
2	-17.902	0.678	0.000	
6	-15.960	0.000	0.000	

Stress Report for Loadcase 4.

Member Design Forces (0 Axial & Moment: NO stress check needed)

Panel		Start		Midpanel		End	
		Axial	Moment	Axial	Moment	Axial	Moment
Верх. пояс							
T1	1-2	0.000	0.000	0.403	-0.151	0.807	-0.603
T1	2-3	-31.866	-0.830	-30.667	-1.191	-29.927	-1.587
T1	3-4	-20.705	-1.587	-20.565	1.507	-20.426	-0.694
T1	4-5	-20.387	-0.455	-20.506	1.105	-20.626	-1.096
T1	5-6	-27.437	-1.096	-28.032	-0.805	-29.098	-0.542
T1	6-7	0.691	-0.533	0.346	-0.133	0.000	0.000
Нижний пояс							
B1	2-8	28.836	0.000	28.836	0.507	28.836	-0.597
B1	8-9	25.850	-0.597	25.850	-0.098	25.850	0.252
B2	9-6	25.850	0.252	25.850	0.507	25.850	0.000
Диагонали							
W1	3-8	-11.724	0.000	-11.724	0.000	-11.724	0.000
W2	8-4	9.082	0.000	9.082	0.000	9.082	0.000
W1	8-5	-8.522	0.000	-8.522	0.000	-8.522	0.000

Stress Report for Loadcase 4.

Panel	Lumber			P/AF	M/ZF	CSI	SSI	Purlin Space	Ke
Верх. пояс									
T1	1-2	50x150	SI-G	0.01	0.24	0.25	0.17	0	2.00
T1	2-3	50x150	SI-G	0.32	0.62	0.94	0.35	0	0.39
T1	3-4	50x150	SI-G	0.23	0.62	0.85	0.39	0	0.60
T1	4-5	50x150	SI-G	0.24	0.43	0.68	0.28	0	0.35
T1	5-6	50x150	SI-G	0.29	0.43	0.72	0.25	0	0.41
T1	6-7	50x150	SI-G	0.01	0.21	0.22	0.15	0	2.00
Нижний пояс									
B1	2-8	50x150	SI-G	0.48	0.23	0.71	0.08	0	0.47
B1	8-9	50x150	SI-G	0.43	0.23	0.66	0.08	0	0.60
B2	9-6	50x150	SI-G	0.43	0.20	0.63	0.05	0	0.60
Диагонали									
W1	3-8	50x150	SI-G	0.17	0.00	0.17	0.00	0	1.00
W2	8-4	50x150	SI-G	0.15	0.00	0.15	0.00	0	1.00
W1	8-5	50x150	SI-G	0.13	0.00	0.13	0.00	0	1.00

Jnt Id.	Lumber	P/AF	M/ZF	CSI	SSI	Purlin Space	Ke
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LOADCASE 5: Lumber DOL = 3.00 1.0ПН+1.4Вспр

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Truss Spacing: 900

Loading (kN/m) (Global Reference Frame)

Panel		Start		End		
		Vert./ Normal	Horiz./ Parallel	Vert./ Normal	Horiz./ Parallel	
Верх. пояс						
T1	1-2	0.364	-0.020	0.364	-0.020	proj.
T1	2-10	0.222	-0.082	0.222	-0.082	proj.
T1	10-11	0.222	-0.082	0.222	-0.082	proj.
T1	11-3	0.222	-0.082	0.222	-0.082	proj.
T1	3-12	0.222	-0.081	0.222	-0.081	proj.
T1	12-4	0.062	-0.149	0.062	-0.149	proj.
T1	4-13	0.560	-0.062	0.560	-0.062	proj.
T1	13-5	0.560	-0.062	0.560	-0.062	proj.
T1	5-14	0.561	-0.063	0.561	-0.063	proj.
T1	14-15	0.561	-0.063	0.561	-0.063	proj.
T1	15-6	0.637	-0.096	0.637	-0.096	proj.
T1	6-7	0.252	0.066	0.252	0.066	proj.
Нижний пояс						
B1	2-8	0.293	0.000	0.293	0.000	proj.
B1	8-9	0.293	0.000	0.293	0.000	proj.
B2	9-6	0.293	0.000	0.293	0.000	proj.

Диагонали
Note: Loads not marked projected, (proj), are loads normal to and parallel to the member.

Member Forces (Local Reference Frame)

		Panel Start			Panel End			Max.
		Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Panel Moment
Верх. пояс								
T1	1-2	0.000	0.000	0.000	0.197	0.094	-0.064	-0.016
T1	2-3	0.204	-4.488	0.064	0.139	-4.164	0.007	0.068
T1	3-4	0.030	-4.002	-0.007	0.003	-3.646	0.000	0.010
T1	4-5	0.480	-3.444	0.000	0.710	-3.800	-0.275	0.232
T1	5-6	0.688	-5.221	0.275	0.507	-5.546	-0.041	-0.204
T1	6-7	0.125	0.094	0.041	0.000	0.000	0.000	-0.010
Нижний пояс								
B1	2-8	0.469	3.299	0.000	0.763	3.299	-0.617	0.375
B1	8-9	0.763	4.940	0.617	-0.382	4.940	0.127	-0.183
B2	9-6	0.382	4.940	-0.127	0.469	4.940	0.000	0.375
Диагонали								
W1	3-8	0.000	-0.226	0.000	0.000	-0.226	0.000	0.000
W2	8-4	0.000	2.326	0.000	0.000	2.326	0.000	0.000
W1	8-5	0.000	-1.985	0.000	0.000	-1.985	0.000	0.000

Reactions (Global Reference Frame)

Joint Id.	Vertical (kN)	Horiz. (kN)	Moment (kNm)	Bearing Width (mm)
2	-2.641	-0.759	0.000	
6	-3.268	0.000	0.000	

Stress Report for Loadcase 5.

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Member Design Forces (0 Axial & Moment: NO stress check needed)

Panel		Start		Midpanel		End	
		Axial	Moment	Axial	Moment	Axial	Moment
Верх. пояс							
T1	1-2	0.000	0.000	0.047	-0.016	0.094	-0.064
T1	2-3	-4.488	-0.064	-4.295	0.068	-4.261	0.064
T1	3-4	-4.002	0.010	-3.974	0.010	-3.969	0.010
T1	4-5	-3.767	-0.131	-3.784	0.232	-3.800	-0.275
T1	5-6	-5.319	-0.275	-5.414	-0.204	-5.546	-0.139
T1	6-7	0.094	-0.041	0.047	-0.010	0.000	0.000
Нижний пояс							
B1	2-8	3.299	0.000	3.299	0.375	3.299	-0.617
B1	8-9	4.940	-0.617	4.940	-0.183	4.940	0.127
B2	9-6	4.940	0.127	4.940	0.375	4.940	0.000
Диагонали							
W1	3-8	-0.226	0.000	-0.226	0.000	-0.226	0.000
W2	8-4	2.326	0.000	2.326	0.000	2.326	0.000
W1	8-5	-1.985	0.000	-1.985	0.000	-1.985	0.000

Stress Report for Loadcase 5.

Panel	Lumber			P/AF	M/ZF	CSI	SSI	Purlin Space	Ke
Верх. пояс									
T1	1-2	50x150	SI-G	0.00	0.03	0.03	0.02	0	2.00
T1	2-3	50x150	SI-G	0.05	0.03	0.08	0.02	0	0.39
T1	3-4	50x150	SI-G	0.05	0.00	0.05	0.00	0	0.60
T1	4-5	50x150	SI-G	0.04	0.11	0.15	0.06	0	0.35
T1	5-6	50x150	SI-G	0.06	0.11	0.16	0.06	0	0.41
T1	6-7	50x150	SI-G	0.00	0.02	0.02	0.01	0	2.00
Нижний пояс									
B1	2-8	50x150	SI-G	0.05	0.24	0.30	0.07	0	0.47
B1	8-9	50x150	SI-G	0.08	0.24	0.32	0.07	0	0.60
B2	9-6	50x150	SI-G	0.08	0.15	0.23	0.04	0	0.60
Диагонали									
W1	3-8	50x150	SI-G	0.00	0.00	0.00	0.00	0	1.00
W2	8-4	50x150	SI-G	0.04	0.00	0.04	0.00	0	1.00
W1	8-5	50x150	SI-G	0.03	0.00	0.03	0.00	0	1.00

Jnt Id.	Lumber	P/AF	M/ZF	CSI	SSI	Purlin Space	Ke
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LOADCASE 6: Lumber DOL = 3.00 1.0ПН+1.4Всл

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Truss Spacing: 900

Loading (kN/m) (Global Reference Frame)

Panel		Start		End		
		Vert./ Normal	Horiz./ Parallel	Vert./ Normal	Horiz./ Parallel	
Верх. пояс						
T1	1-2	0.252	-0.066	0.252	-0.066	proj.
T1	2-10	0.637	0.096	0.637	0.096	proj.
T1	10-11	0.561	0.063	0.561	0.063	proj.
T1	11-3	0.561	0.063	0.561	0.063	proj.
T1	3-12	0.560	0.062	0.560	0.062	proj.
T1	12-4	0.560	0.062	0.560	0.062	proj.
T1	4-13	0.062	0.149	0.062	0.149	proj.
T1	13-5	0.222	0.081	0.222	0.081	proj.
T1	5-14	0.222	0.082	0.222	0.082	proj.
T1	14-15	0.222	0.082	0.222	0.082	proj.
T1	15-6	0.222	0.082	0.222	0.082	proj.
T1	6-7	0.364	0.020	0.364	0.020	proj.
Нижний пояс						
B1	2-8	0.293	0.000	0.293	0.000	proj.
B1	8-9	0.293	0.000	0.293	0.000	proj.
B2	9-6	0.293	0.000	0.293	0.000	proj.

Диагонали

Note: Loads not marked projected, (proj), are loads normal to and parallel to the member.

Member Forces (Local Reference Frame)

		Panel Start			Panel End			Max.
		Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Panel Moment
Верх. пояс								
T1	1-2	0.000	0.000	0.000	0.125	0.094	-0.041	-0.010
T1	2-3	0.509	-5.540	0.041	0.686	-5.215	-0.271	-0.200
T1	3-4	0.708	-3.798	0.271	0.482	-3.442	0.000	0.233
T1	4-5	0.005	-3.644	0.000	0.028	-4.000	0.011	0.014
T1	5-6	0.137	-4.157	-0.011	0.207	-4.482	-0.064	0.071
T1	6-7	0.197	0.094	0.064	0.000	0.000	0.000	-0.016
Нижний пояс								
B1	2-8	0.470	5.692	0.000	0.763	5.692	-0.615	0.376
B1	8-9	0.763	4.052	0.615	-0.381	4.052	0.129	-0.181
B2	9-6	0.381	4.052	-0.129	0.470	4.052	0.000	0.376
Диагонали								
W1	3-8	0.000	-1.979	0.000	0.000	-1.979	0.000	0.000
W2	8-4	0.000	2.321	0.000	0.000	2.321	0.000	0.000
W1	8-5	0.000	-0.220	0.000	0.000	-0.220	0.000	0.000

Reactions (Global Reference Frame)

Joint Id.	Vertical (kN)	Horiz. (kN)	Moment (kNm)	Bearing Width (mm)
2	-3.268	0.759	0.000	
6	-2.641	0.000	0.000	

Stress Report for Loadcase 6.

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Member Design Forces (0 Axial & Moment: NO stress check needed)

Panel		Start		Midpanel		End	
		Axial	Moment	Axial	Moment	Axial	Moment
Верх. пояс							
T1	1-2	0.000	0.000	0.047	-0.010	0.094	-0.041
T1	2-3	-5.540	-0.135	-5.407	-0.200	-5.312	-0.271
T1	3-4	-3.798	-0.271	-3.781	0.233	-3.765	-0.127
T1	4-5	-3.967	0.014	-3.974	0.014	-4.000	0.014
T1	5-6	-4.254	0.067	-4.287	0.071	-4.482	0.067
T1	6-7	0.094	-0.064	0.047	-0.016	0.000	0.000
Нижний пояс							
B1	2-8	5.692	0.000	5.692	0.376	5.692	-0.615
B1	8-9	4.052	-0.615	4.052	-0.181	4.052	0.129
B2	9-6	4.052	0.129	4.052	0.376	4.052	0.000
Диагонали							
W1	3-8	-1.979	0.000	-1.979	0.000	-1.979	0.000
W2	8-4	2.321	0.000	2.321	0.000	2.321	0.000
W1	8-5	-0.220	0.000	-0.220	0.000	-0.220	0.000

Stress Report for Loadcase 6.

Panel	Lumber			P/AF	M/ZF	CSI	SSI	Purlin Space	Ke
Верх. пояс									
T1	1-2	50x150	SI-G	0.00	0.02	0.02	0.01	0	2.00
T1	2-3	50x150	SI-G	0.06	0.11	0.16	0.06	0	0.39
T1	3-4	50x150	SI-G	0.04	0.11	0.15	0.06	0	0.60
T1	4-5	50x150	SI-G	0.05	0.01	0.06	0.00	0	0.35
T1	5-6	50x150	SI-G	0.05	0.03	0.08	0.02	0	0.41
T1	6-7	50x150	SI-G	0.00	0.03	0.03	0.02	0	2.00
Нижний пояс									
B1	2-8	50x150	SI-G	0.09	0.24	0.33	0.07	0	0.47
B1	8-9	50x150	SI-G	0.07	0.24	0.31	0.07	0	0.60
B2	9-6	50x150	SI-G	0.07	0.15	0.21	0.04	0	0.60
Диагонали									
W1	3-8	50x150	SI-G	0.03	0.00	0.03	0.00	0	1.00
W2	8-4	50x150	SI-G	0.04	0.00	0.04	0.00	0	1.00
W1	8-5	50x150	SI-G	0.00	0.00	0.00	0.00	0	1.00

Jnt Id.	Lumber	P/AF	M/ZF	CSI	SSI	Purlin Space	Ke
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LOADCASE 7: Lumber DOL = 3.00 1.2_{ПН}+1.4*0.5_{μ1}CH_П+1.25_{ВН}

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Truss Spacing: 900

Loading (kN/m) (Global Reference Frame)

Panel		Start		End		
		Vert./ Normal	Horiz./ Parallel	Vert./ Normal	Horiz./ Parallel	
Верх. пояс						
T1	1-2	0.494	0.000	0.494	0.000	proj.
T1	2-10	0.497	0.000	0.497	0.000	proj.
T1	10-11	0.497	0.000	0.497	0.000	proj.
T1	11-3	0.497	0.000	0.497	0.000	proj.
T1	3-12	0.496	0.000	0.496	0.000	proj.
T1	12-4	0.496	0.000	0.496	0.000	proj.
T1	4-13	1.756	0.000	1.756	0.000	proj.
T1	13-5	1.756	0.000	1.756	0.000	proj.
T1	5-14	1.757	0.000	1.757	0.000	proj.
T1	14-15	1.757	0.000	1.757	0.000	proj.
T1	15-6	1.757	0.000	1.757	0.000	proj.
T1	6-7	1.754	0.000	1.754	0.000	proj.
Нижний пояс						
B1	2-8	0.352	0.000	0.352	0.000	proj.
B1	8-9	0.352	0.000	0.352	0.000	proj.
B2	9-6	0.352	0.000	0.352	0.000	proj.

Диагонали

Note: Loads not marked projected, (proj), are loads normal to and parallel to the member.

Member Forces (Local Reference Frame)

		Panel Start			Panel End			Max.
		Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Panel Moment
Верх. пояс								
T1	1-2	0.000	0.000	0.000	0.274	0.113	-0.089	-0.022
T1	2-3	0.424	-9.910	0.089	0.488	-9.520	-0.158	0.125
T1	3-4	0.569	-8.462	0.158	0.437	-8.035	0.000	0.227
T1	4-5	1.465	-7.598	0.000	2.095	-9.110	-0.754	0.722
T1	5-6	1.814	-13.090	0.754	1.411	-14.469	-0.316	-0.568
T1	6-7	0.973	0.402	0.316	0.000	0.000	0.000	-0.079
Нижний пояс								
B1	2-8	0.572	8.945	0.000	0.907	8.945	-0.703	0.465
B1	8-9	0.907	12.748	0.703	-0.449	12.748	0.178	-0.188
B2	9-6	0.449	12.748	-0.178	0.572	12.748	0.000	0.465
Диагонали								
W1	3-8	0.000	-1.478	0.000	0.000	-1.478	0.000	0.000
W2	8-4	0.000	4.357	0.000	0.000	4.357	0.000	0.000
W1	8-5	0.000	-5.557	0.000	0.000	-5.557	0.000	0.000

Reactions (Global Reference Frame)

Joint Id.	Vertical (kN)	Horiz. (kN)	Moment (kNm)	Bearing Width (mm)
2	-5.156	0.000	0.000	
6	-8.612	0.000	0.000	

Stress Report for Loadcase 7.

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Member Design Forces (0 Axial & Moment: NO stress check needed)

Panel		Start		Midpanel		End	
		Axial	Moment	Axial	Moment	Axial	Moment
Верх. пояс							
T1	1-2	0.000	0.000	0.057	-0.022	0.113	-0.089
T1	2-3	-9.910	-0.089	-9.729	0.125	-9.637	-0.158
T1	3-4	-8.462	-0.158	-8.442	0.227	-8.422	-0.043
T1	4-5	-8.970	-0.328	-9.040	0.722	-9.110	-0.754
T1	5-6	-13.502	-0.754	-13.866	-0.568	-14.469	-0.399
T1	6-7	0.402	-0.316	0.201	-0.079	0.000	0.000
Нижний пояс							
B1	2-8	8.945	0.000	8.945	0.465	8.945	-0.703
B1	8-9	12.748	-0.703	12.748	-0.188	12.748	0.178
B2	9-6	12.748	0.178	12.748	0.465	12.748	0.000
Диагонали							
W1	3-8	-1.478	0.000	-1.478	0.000	-1.478	0.000
W2	8-4	4.357	0.000	4.357	0.000	4.357	0.000
W1	8-5	-5.557	0.000	-5.557	0.000	-5.557	0.000

Stress Report for Loadcase 7.

Panel	Lumber		P/AF	M/ZF	CSI	SSI	Purlin Space	Ke
Верх. пояс								
T1	1-2	50x150 SI-G	0.00	0.03	0.04	0.02	0	2.00
T1	2-3	50x150 SI-G	0.11	0.05	0.16	0.04	0	0.39
T1	3-4	50x150 SI-G	0.10	0.09	0.19	0.05	0	0.60
T1	4-5	50x150 SI-G	0.10	0.29	0.39	0.19	0	0.35
T1	5-6	50x150 SI-G	0.14	0.29	0.44	0.16	0	0.41
T1	6-7	50x150 SI-G	0.01	0.12	0.13	0.09	0	2.00
Нижний пояс								
B1	2-8	50x150 SI-G	0.15	0.27	0.42	0.08	0	0.47
B1	8-9	50x150 SI-G	0.21	0.27	0.49	0.08	0	0.60
B2	9-6	50x150 SI-G	0.21	0.18	0.39	0.05	0	0.60
Диагонали								
W1	3-8	50x150 SI-G	0.02	0.00	0.02	0.00	0	1.00
W2	8-4	50x150 SI-G	0.07	0.00	0.07	0.00	0	1.00
W1	8-5	50x150 SI-G	0.08	0.00	0.08	0.00	0	1.00

Jnt Id.	Lumber	P/AF	M/ZF	CSI	SSI	Purlin Space	Ke
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LOADCASE 8: Lumber DOL = 3.00 1.2ПН+1.4*0.5μ1CH_Л +1.25BH

Truss Spacing: 900

Loading (kN/m) (Global Reference Frame)

Panel		Start		End		
		Vert./ Normal	Horiz./ Parallel	Vert./ Normal	Horiz./ Parallel	
Верх. пояс						
T1	1-2	1.754	0.000	1.754	0.000	proj.
T1	2-10	1.757	0.000	1.757	0.000	proj.
T1	10-11	1.757	0.000	1.757	0.000	proj.
T1	11-3	1.757	0.000	1.757	0.000	proj.
T1	3-12	1.756	0.000	1.756	0.000	proj.
T1	12-4	1.756	0.000	1.756	0.000	proj.
T1	4-13	0.496	0.000	0.496	0.000	proj.
T1	13-5	0.496	0.000	0.496	0.000	proj.
T1	5-14	0.497	0.000	0.497	0.000	proj.
T1	14-15	0.497	0.000	0.497	0.000	proj.
T1	15-6	0.497	0.000	0.497	0.000	proj.
T1	6-7	0.494	0.000	0.494	0.000	proj.
Нижний пояс						
B1	2-8	0.352	0.000	0.352	0.000	proj.
B1	8-9	0.352	0.000	0.352	0.000	proj.
B2	9-6	0.352	0.000	0.352	0.000	proj.

Note: Loads not marked projected, (proj), are loads normal to and parallel to the member.

Member Forces (Local Reference Frame)

		Panel Start			Panel End			Max.
		Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Panel Moment
Верх. пояс								
T1	1-2	0.000	0.000	0.000	0.973	0.402	-0.316	-0.079
T1	2-3	1.411	-14.469	0.316	1.814	-13.090	-0.754	-0.568
T1	3-4	2.095	-9.110	0.754	1.465	-7.598	0.000	0.722
T1	4-5	0.437	-8.035	0.000	0.569	-8.462	-0.158	0.227
T1	5-6	0.488	-9.520	0.158	0.424	-9.910	-0.089	0.125
T1	6-7	0.274	0.113	0.089	0.000	0.000	0.000	-0.022
Нижний пояс								
B1	2-8	0.572	12.748	0.000	0.907	12.748	-0.703	0.465
B1	8-9	0.907	8.945	0.703	-0.449	8.945	0.178	-0.188
B2	9-6	0.449	8.945	-0.178	0.572	8.945	0.000	0.465
Диагонали								
W1	3-8	0.000	-5.557	0.000	0.000	-5.557	0.000	0.000
W2	8-4	0.000	4.357	0.000	0.000	4.357	0.000	0.000
W1	8-5	0.000	-1.478	0.000	0.000	-1.478	0.000	0.000

Reactions (Global Reference Frame)

Joint Id.	Vertical (kN)	Horiz. (kN)	Moment (kNm)	Bearing Width (mm)
2	-8.612	0.000	0.000	
6	-5.156	0.000	0.000	

Stress Report for Loadcase 8.

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Member Design Forces (0 Axial & Moment: NO stress check needed)

Panel		Start		Midpanel		End	
		Axial	Moment	Axial	Moment	Axial	Moment
Верх. пояс							
T1	1-2	0.000	0.000	0.201	-0.079	0.402	-0.316
T1	2-3	-14.469	-0.399	-13.866	-0.568	-13.502	-0.754
T1	3-4	-9.110	-0.754	-9.040	0.722	-8.970	-0.328
T1	4-5	-8.422	-0.043	-8.442	0.227	-8.462	-0.158
T1	5-6	-9.637	-0.158	-9.729	0.125	-9.910	-0.089
T1	6-7	0.113	-0.089	0.057	-0.022	0.000	0.000
Нижний пояс							
B1	2-8	12.748	0.000	12.748	0.465	12.748	-0.703
B1	8-9	8.945	-0.703	8.945	-0.188	8.945	0.178
B2	9-6	8.945	0.178	8.945	0.465	8.945	0.000
Диагонали							
W1	3-8	-5.557	0.000	-5.557	0.000	-5.557	0.000
W2	8-4	4.357	0.000	4.357	0.000	4.357	0.000
W1	8-5	-1.478	0.000	-1.478	0.000	-1.478	0.000

Stress Report for Loadcase 8.

Panel	Lumber			P/AF	M/ZF	CSI	SSI	Purlin Space	Ke
Верх. пояс									
T1	1-2	50x150	SI-G	0.01	0.12	0.13	0.09	0	2.00
T1	2-3	50x150	SI-G	0.14	0.29	0.44	0.16	0	0.39
T1	3-4	50x150	SI-G	0.10	0.29	0.39	0.19	0	0.60
T1	4-5	50x150	SI-G	0.10	0.09	0.19	0.05	0	0.35
T1	5-6	50x150	SI-G	0.11	0.05	0.16	0.04	0	0.41
T1	6-7	50x150	SI-G	0.00	0.03	0.04	0.02	0	2.00
Нижний пояс									
B1	2-8	50x150	SI-G	0.21	0.27	0.49	0.08	0	0.47
B1	8-9	50x150	SI-G	0.15	0.27	0.42	0.08	0	0.60
B2	9-6	50x150	SI-G	0.15	0.18	0.33	0.05	0	0.60
Диагонали									
W1	3-8	50x150	SI-G	0.08	0.00	0.08	0.00	0	1.00
W2	8-4	50x150	SI-G	0.07	0.00	0.07	0.00	0	1.00
W1	8-5	50x150	SI-G	0.02	0.00	0.02	0.00	0	1.00

Jnt Id.	Lumber	P/AF	M/ZF	CSI	SSI	Purlin Space	Ke
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LOADCASE 9: Lumber DOL = 3.00 1.2ПН+1.4*0.5μ1CH_П+1.25Вспр+1.25ВН

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Truss Spacing: 900

Loading (kN/m) (Global Reference Frame)

Panel		Start		End		
		Vert./ Normal	Horiz./ Parallel	Vert./ Normal	Horiz./ Parallel	
Верх. пояс						
T1	1-2	0.451	-0.018	0.451	-0.018	proj.
T1	2-10	0.326	-0.073	0.326	-0.073	proj.
T1	10-11	0.326	-0.073	0.326	-0.073	proj.
T1	11-3	0.326	-0.073	0.326	-0.073	proj.
T1	3-12	0.325	-0.073	0.325	-0.073	proj.
T1	12-4	0.182	-0.133	0.182	-0.133	proj.
T1	4-13	1.887	-0.056	1.887	-0.056	proj.
T1	13-5	1.887	-0.056	1.887	-0.056	proj.
T1	5-14	1.888	-0.056	1.888	-0.056	proj.
T1	14-15	1.888	-0.056	1.888	-0.056	proj.
T1	15-6	1.956	-0.085	1.956	-0.085	proj.
T1	6-7	1.611	0.059	1.611	0.059	proj.
Нижний пояс						
B1	2-8	0.352	0.000	0.352	0.000	proj.
B1	8-9	0.352	0.000	0.352	0.000	proj.
B2	9-6	0.352	0.000	0.352	0.000	proj.

Диагонали

Note: Loads not marked projected, (proj), are loads normal to and parallel to the member.

Member Forces (Local Reference Frame)

		Panel Start			Panel End			Max.
		Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Panel Moment
Верх. пояс								
T1	1-2	0.000	0.000	0.000	0.246	0.113	-0.080	-0.020
T1	2-3	0.301	-8.957	0.080	0.239	-8.568	-0.012	0.103
T1	3-4	0.163	-8.180	0.012	0.124	-7.753	0.000	0.072
T1	4-5	1.578	-7.136	0.000	2.297	-8.647	-0.860	0.769
T1	5-6	2.056	-13.081	0.860	1.558	-14.461	-0.286	-0.649
T1	6-7	0.880	0.402	0.286	0.000	0.000	0.000	-0.071
Нижний пояс								
B1	2-8	0.571	7.450	0.000	0.908	7.450	-0.707	0.463
B1	8-9	0.908	12.718	0.707	-0.450	12.718	0.175	-0.192
B2	9-6	0.450	12.718	-0.175	0.571	12.718	0.000	0.463
Диагонали								
W1	3-8	0.000	-0.542	0.000	0.000	-0.542	0.000	0.000
W2	8-4	0.000	4.250	0.000	0.000	4.250	0.000	0.000
W1	8-5	0.000	-6.192	0.000	0.000	-6.192	0.000	0.000

Reactions (Global Reference Frame)

Joint Id.	Vertical (kN)	Horiz. (kN)	Moment (kNm)	Bearing Width (mm)
2	-4.642	-0.678	0.000	
6	-8.658	0.000	0.000	

Stress Report for Loadcase 9.

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Member Design Forces (0 Axial & Moment: NO stress check needed)

Panel		Start		Midpanel		End	
		Axial	Moment	Axial	Moment	Axial	Moment
Верх. пояс							
T1	1-2	0.000	0.000	0.057	-0.020	0.113	-0.080
T1	2-3	-8.957	0.091	-8.740	0.103	-8.684	0.091
T1	3-4	-8.180	0.018	-8.160	0.072	-8.140	0.018
T1	4-5	-8.508	-0.392	-8.578	0.769	-8.647	-0.860
T1	5-6	-13.494	-0.860	-13.873	-0.649	-14.461	-0.456
T1	6-7	0.402	-0.286	0.201	-0.071	0.000	0.000
Нижний пояс							
B1	2-8	7.450	0.000	7.450	0.463	7.450	-0.707
B1	8-9	12.718	-0.707	12.718	-0.192	12.718	0.175
B2	9-6	12.718	0.175	12.718	0.463	12.718	0.000
Диагонали							
W1	3-8	-0.542	0.000	-0.542	0.000	-0.542	0.000
W2	8-4	4.250	0.000	4.250	0.000	4.250	0.000
W1	8-5	-6.192	0.000	-6.192	0.000	-6.192	0.000

Stress Report for Loadcase 9.

Panel	Lumber		P/AF	M/ZF	CSI	SSI	Purlin Space	Ke
Верх. пояс								
T1	1-2	50x150 SI-G	0.00	0.03	0.03	0.02	0	2.00
T1	2-3	50x150 SI-G	0.11	0.04	0.15	0.03	0	0.39
T1	3-4	50x150 SI-G	0.10	0.03	0.13	0.01	0	0.60
T1	4-5	50x150 SI-G	0.09	0.34	0.43	0.21	0	0.35
T1	5-6	50x150 SI-G	0.14	0.34	0.48	0.18	0	0.41
T1	6-7	50x150 SI-G	0.01	0.11	0.12	0.08	0	2.00
Нижний пояс								
B1	2-8	50x150 SI-G	0.12	0.28	0.40	0.08	0	0.47
B1	8-9	50x150 SI-G	0.21	0.28	0.49	0.08	0	0.60
B2	9-6	50x150 SI-G	0.21	0.18	0.39	0.05	0	0.60
Диагонали								
W1	3-8	50x150 SI-G	0.01	0.00	0.01	0.00	0	1.00
W2	8-4	50x150 SI-G	0.07	0.00	0.07	0.00	0	1.00
W1	8-5	50x150 SI-G	0.09	0.00	0.09	0.00	0	1.00

Jnt Id.	Lumber	P/AF	M/ZF	CSI	SSI	Purlin Space	Ke
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LOADCASE 10: Lumber DOL = 3.00 1.2ПН+1.4*0.5μ1CH_Л +1.4Всл+1.25ВН

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Truss Spacing: 900

Loading (kN/m) (Global Reference Frame)

Panel		Start		End		
		Vert./ Normal	Horiz./ Parallel	Vert./ Normal	Horiz./ Parallel	
Верх. пояс						
T1	1-2	1.611	-0.059	1.611	-0.059	proj.
T1	2-10	1.956	0.085	1.956	0.085	proj.
T1	10-11	1.888	0.056	1.888	0.056	proj.
T1	11-3	1.888	0.056	1.888	0.056	proj.
T1	3-12	1.887	0.056	1.887	0.056	proj.
T1	12-4	1.887	0.056	1.887	0.056	proj.
T1	4-13	0.182	0.133	0.182	0.133	proj.
T1	13-5	0.325	0.073	0.325	0.073	proj.
T1	5-14	0.326	0.073	0.326	0.073	proj.
T1	14-15	0.326	0.073	0.326	0.073	proj.
T1	15-6	0.326	0.073	0.326	0.073	proj.
T1	6-7	0.451	0.018	0.451	0.018	proj.
Нижний пояс						
B1	2-8	0.352	0.000	0.352	0.000	proj.
B1	8-9	0.352	0.000	0.352	0.000	proj.
B2	9-6	0.352	0.000	0.352	0.000	proj.

Диагонали

Note: Loads not marked projected, (proj), are loads normal to and parallel to the member.

Member Forces (Local Reference Frame)

		Panel Start			Panel End			Max.
		Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Panel Moment
Верх. пояс								
T1	1-2	0.000	0.000	0.000	0.880	0.402	-0.286	-0.071
T1	2-3	1.560	-14.455	0.286	2.054	-13.076	-0.856	-0.645
T1	3-4	2.295	-8.645	0.856	1.580	-7.134	0.000	0.771
T1	4-5	0.126	-7.751	0.000	0.161	-8.178	-0.008	0.074
T1	5-6	0.237	-8.562	0.008	0.303	-8.952	-0.080	0.105
T1	6-7	0.246	0.113	0.080	0.000	0.000	0.000	-0.020
Нижний пояс								
B1	2-8	0.572	13.390	0.000	0.907	13.390	-0.705	0.464
B1	8-9	0.907	8.122	0.705	-0.450	8.122	0.177	-0.190
B2	9-6	0.450	8.122	-0.177	0.572	8.122	0.000	0.464
Диагонали								
W1	3-8	0.000	-6.187	0.000	0.000	-6.187	0.000	0.000
W2	8-4	0.000	4.246	0.000	0.000	4.246	0.000	0.000
W1	8-5	0.000	-0.537	0.000	0.000	-0.537	0.000	0.000

Reactions (Global Reference Frame)

Joint Id.	Vertical (kN)	Horiz. (kN)	Moment (kNm)	Bearing Width (mm)
2	-8.658	0.678	0.000	
6	-4.642	0.000	0.000	

Stress Report for Loadcase 10.

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Member Design Forces (0 Axial & Moment: NO stress check needed)

Panel		Start		Midpanel		End	
		Axial	Moment	Axial	Moment	Axial	Moment
Верх. пояс							
T1	1-2	0.000	0.000	0.201	-0.071	0.402	-0.286
T1	2-3	-14.455	-0.453	-13.867	-0.645	-13.488	-0.856
T1	3-4	-8.645	-0.856	-8.575	0.771	-8.506	-0.389
T1	4-5	-8.138	0.022	-8.158	0.074	-8.178	0.022
T1	5-6	-8.679	0.094	-8.733	0.105	-8.952	0.094
T1	6-7	0.113	-0.080	0.057	-0.020	0.000	0.000
Нижний пояс							
B1	2-8	13.390	0.000	13.390	0.464	13.390	-0.705
B1	8-9	8.122	-0.705	8.122	-0.190	8.122	0.177
B2	9-6	8.122	0.177	8.122	0.464	8.122	0.000
Диагонали							
W1	3-8	-6.187	0.000	-6.187	0.000	-6.187	0.000
W2	8-4	4.246	0.000	4.246	0.000	4.246	0.000
W1	8-5	-0.537	0.000	-0.537	0.000	-0.537	0.000

Stress Report for Loadcase 10.

Panel	Lumber			P/AF	M/ZF	CSI	SSI	Purlin Space	Ke
Верх. пояс									
T1	1-2	50x150	SI-G	0.01	0.11	0.12	0.08	0	2.00
T1	2-3	50x150	SI-G	0.14	0.33	0.48	0.18	0	0.39
T1	3-4	50x150	SI-G	0.09	0.33	0.43	0.21	0	0.60
T1	4-5	50x150	SI-G	0.10	0.03	0.13	0.01	0	0.35
T1	5-6	50x150	SI-G	0.11	0.04	0.15	0.03	0	0.41
T1	6-7	50x150	SI-G	0.00	0.03	0.03	0.02	0	2.00
Нижний пояс									
B1	2-8	50x150	SI-G	0.22	0.28	0.50	0.08	0	0.47
B1	8-9	50x150	SI-G	0.13	0.28	0.41	0.08	0	0.60
B2	9-6	50x150	SI-G	0.13	0.18	0.32	0.05	0	0.60
Диагонали									
W1	3-8	50x150	SI-G	0.09	0.00	0.09	0.00	0	1.00
W2	8-4	50x150	SI-G	0.07	0.00	0.07	0.00	0	1.00
W1	8-5	50x150	SI-G	0.01	0.00	0.01	0.00	0	1.00

Jnt Id.	Lumber	P/AF	M/ZF	CSI	SSI	Purlin Space	Ke
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LOADCASE 13: Lumber DOL = 3.00 1.0ПН+1.4W ътнт F-G

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Truss Spacing: 900

Loading (kN/m) (Global Reference Frame)

Panel		Start		End		
		Vert./ Normal	Horiz./ Parallel	Vert./ Normal	Horiz./ Parallel	
Верх. пояс						
T1	1-2	0.457	0.018	0.457	0.018	proj.
T1	2-10	0.075	-0.145	0.075	-0.145	proj.
T1	10-11	0.075	-0.145	0.075	-0.145	proj.
T1	11-3	0.075	-0.145	0.075	-0.145	proj.
T1	3-12	0.075	-0.144	0.075	-0.144	proj.
T1	12-4	0.075	-0.144	0.075	-0.144	proj.
T1	4-13	0.075	0.144	0.075	0.144	proj.
T1	13-5	0.075	0.144	0.075	0.144	proj.
T1	5-14	0.075	0.145	0.075	0.145	proj.
T1	14-15	0.075	0.145	0.075	0.145	proj.
T1	15-6	0.075	0.145	0.075	0.145	proj.
T1	6-7	0.457	-0.018	0.457	-0.018	proj.
Нижний пояс						
B1	2-8	0.293	0.000	0.293	0.000	proj.
B1	8-9	0.293	0.000	0.293	0.000	proj.
B2	9-6	0.293	0.000	0.293	0.000	proj.

Диагонали
Note: Loads not marked projected, (proj), are loads normal to and parallel to the member.

Member Forces (Local Reference Frame)

		Panel Start			Panel End			Max.
		Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Panel Moment (kNm)
Верх. пояс								
T1	1-2	0.000	0.000	0.000	0.257	0.094	-0.084	0.000
T1	2-3	0.067	-2.594	0.084	-0.043	-2.269	0.036	-0.036
T1	3-4	-0.001	-2.320	-0.036	0.029	-1.964	0.000	0.036
T1	4-5	0.029	-1.964	0.000	-0.001	-2.320	0.036	0.036
T1	5-6	-0.043	-2.269	-0.036	0.067	-2.594	-0.084	-0.036
T1	6-7	0.257	0.094	0.084	0.000	0.000	0.000	-0.021
Нижний пояс								
B1	2-8	0.466	2.348	0.000	0.766	2.348	-0.630	0.370
B1	8-9	0.766	2.348	0.630	-0.385	2.348	0.118	-0.194
B2	9-6	0.385	2.348	-0.118	0.466	2.348	0.000	0.370
Диагонали								
W1	3-8	0.000	0.071	0.000	0.000	0.071	0.000	0.000
W2	8-4	0.000	1.482	0.000	0.000	1.482	0.000	0.000
W1	8-5	0.000	0.071	0.000	0.000	0.071	0.000	0.000

Reactions (Global Reference Frame)

Joint Id.	Vertical (kN)	Horiz. (kN)	Moment (kNm)	Bearing Width (mm)
2	-1.822	0.000	0.000	
6	-1.822	0.000	0.000	

Stress Report for Loadcase 13.

Member Design Forces (0 Axial & Moment: NO stress check needed)

Panel		Start		Midpanel		End	
		Axial	Moment	Axial	Moment	Axial	Moment
Верх. пояс							
T1	1-2	0.000	0.000	0.000	0.000	0.094	-0.084
T1	2-3	-2.594	-0.084	-2.480	-0.036	-2.366	0.036
T1	3-4	-2.320	0.036	-2.304	0.036	-2.287	0.035
T1	4-5	-2.287	0.035	-2.304	0.036	-2.320	0.036
T1	5-6	-2.366	0.036	-2.480	-0.036	-2.594	-0.084
T1	6-7	0.094	-0.084	0.047	-0.021	0.000	0.000
Нижний пояс							
B1	2-8	2.348	0.000	2.348	0.370	2.348	-0.630
B1	8-9	2.348	-0.630	2.348	-0.194	2.348	0.118
B2	9-6	2.348	0.118	2.348	0.370	2.348	0.000
Диагонали							
W1	3-8	0.071	0.000	0.071	0.000	0.071	0.000
W2	8-4	1.482	0.000	1.482	0.000	1.482	0.000
W1	8-5	0.071	0.000	0.071	0.000	0.071	0.000

Stress Report for Loadcase 13.

Panel	Lumber		P/AF	M/ZF	CSI	SSI	Purlin Space	Ke
Верх. пояс								
T1	1-2	50x150 SI-G	0.00	0.03	0.03	0.02	0	2.00
T1	2-3	50x150 SI-G	0.03	0.03	0.06	0.01	0	0.39
T1	3-4	50x150 SI-G	0.03	0.01	0.04	0.00	0	0.60
T1	4-5	50x150 SI-G	0.03	0.01	0.04	0.00	0	0.35
T1	5-6	50x150 SI-G	0.03	0.03	0.06	0.01	0	0.41
T1	6-7	50x150 SI-G	0.00	0.03	0.03	0.02	0	2.00
Нижний пояс								
B1	2-8	50x150 SI-G	0.04	0.25	0.29	0.07	0	0.47
B1	8-9	50x150 SI-G	0.04	0.25	0.29	0.07	0	0.60
B2	9-6	50x150 SI-G	0.04	0.14	0.18	0.04	0	0.60
Диагонали								
W1	3-8	50x150 SI-G	0.00	0.00	0.00	0.00	0	1.00
W2	8-4	50x150 SI-G	0.02	0.00	0.02	0.00	0	1.00
W1	8-5	50x150 SI-G	0.00	0.00	0.00	0.00	0	1.00

Jnt Id.	Lumber	P/AF	M/ZF	CSI	SSI	Purlin Space	Ke
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LOADCASE 14: Lumber DOL = 3.00 1.0DL+1.4W лбтнт H

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Truss Spacing: 900

Loading (kN/m) (Global Reference Frame)

Panel		Start		End		
		Vert./ Normal	Horiz./ Parallel	Vert./ Normal	Horiz./ Parallel	
Верх. пояс						
T1	1-2	0.319	-0.038	0.319	-0.038	proj.
T1	2-10	-0.158	-0.244	-0.158	-0.244	proj.
T1	10-11	-0.158	-0.244	-0.158	-0.244	proj.
T1	11-3	-0.234	-0.277	-0.234	-0.277	proj.
T1	3-12	-0.235	-0.275	-0.235	-0.275	proj.
T1	12-4	-0.235	-0.275	-0.235	-0.275	proj.
T1	4-13	-0.235	0.275	-0.235	0.275	proj.
T1	13-5	-0.235	0.275	-0.235	0.275	proj.
T1	5-14	-0.234	0.277	-0.234	0.277	proj.
T1	14-15	-0.158	0.244	-0.158	0.244	proj.
T1	15-6	-0.158	0.244	-0.158	0.244	proj.
T1	6-7	0.319	0.038	0.319	0.038	proj.
Нижний пояс						
B1	2-8	0.293	0.000	0.293	0.000	proj.
B1	8-9	0.293	0.000	0.293	0.000	proj.
B2	9-6	0.293	0.000	0.293	0.000	proj.

Диагонали

Note: Loads not marked projected, (proj), are loads normal to and parallel to the member.

Member Forces (Local Reference Frame)

		Panel Start			Panel End			Max.
		Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Panel Moment (kNm)
Верх. пояс								
T1	1-2	0.000	0.000	0.000	0.168	0.094	-0.055	0.000
T1	2-3	-0.131	-0.111	0.055	-0.367	0.214	0.185	0.148
T1	3-4	-0.434	-0.608	-0.185	-0.279	-0.252	0.000	0.139
T1	4-5	-0.279	-0.252	0.000	-0.434	-0.608	0.185	0.139
T1	5-6	-0.367	0.214	-0.185	-0.131	-0.111	-0.055	0.148
T1	6-7	0.168	0.094	0.055	0.000	0.000	0.000	-0.014
Нижний пояс								
B1	2-8	0.463	0.176	0.000	0.770	0.176	-0.645	0.365
B1	8-9	0.770	0.176	0.645	-0.389	0.176	0.108	-0.207
B2	9-6	0.389	0.176	-0.108	0.463	0.176	0.000	0.365
Диагонали								
W1	3-8	0.000	1.148	0.000	0.000	1.148	0.000	0.000
W2	8-4	0.000	0.710	0.000	0.000	0.710	0.000	0.000
W1	8-5	0.000	1.148	0.000	0.000	1.148	0.000	0.000

Reactions (Global Reference Frame)

Joint Id.	Vertical (kN)	Horiz. (kN)	Moment (kNm)	Bearing Width (mm)
2	-0.577	0.000	0.000	
6	-0.577	0.000	0.000	

Stress Report for Loadcase 14.

Member Design Forces (0 Axial & Moment: NO stress check needed)

Panel		Start		Midpanel		End	
		Axial	Moment	Axial	Moment	Axial	Moment
Верх. пояс							
T1	1-2	0.000	0.000	0.000	0.000	0.094	-0.055
T1	2-3	0.182	0.114	0.198	0.148	0.214	0.185
T1	3-4	-0.608	0.185	-0.591	0.139	-0.575	0.097
T1	4-5	-0.575	0.097	-0.591	0.139	-0.608	0.185
T1	5-6	0.214	0.185	0.198	0.148	0.182	0.114
T1	6-7	0.094	-0.055	0.047	-0.014	0.000	0.000
Нижний пояс							
B1	2-8	0.176	0.000	0.176	0.365	0.176	-0.645
B1	8-9	0.176	-0.645	0.176	-0.207	0.176	0.108
B2	9-6	0.176	0.108	0.176	0.365	0.176	0.000
Диагонали							
W1	3-8	1.148	0.000	1.148	0.000	1.148	0.000
W2	8-4	0.710	0.000	0.710	0.000	0.710	0.000
W1	8-5	1.148	0.000	1.148	0.000	1.148	0.000

Stress Report for Loadcase 14.

Panel	Lumber			P/AF	M/ZF	CSI	SSI	Purlin Space	Ke
Верх. пояс									
T1	1-2	50x150	SI-G	0.00	0.02	0.02	0.02	0	2.00
T1	2-3	50x150	SI-G	0.00	0.07	0.08	0.03	0	0.39
T1	3-4	50x150	SI-G	0.01	0.07	0.08	0.04	0	0.60
T1	4-5	50x150	SI-G	0.01	0.07	0.08	0.04	0	0.35
T1	5-6	50x150	SI-G	0.00	0.07	0.08	0.03	0	0.41
T1	6-7	50x150	SI-G	0.00	0.02	0.02	0.02	0	2.00
Нижний пояс									
B1	2-8	50x150	SI-G	0.00	0.25	0.26	0.07	0	0.47
B1	8-9	50x150	SI-G	0.00	0.25	0.26	0.07	0	0.60
B2	9-6	50x150	SI-G	0.00	0.14	0.15	0.04	0	0.60
Диагонали									
W1	3-8	50x150	SI-G	0.02	0.00	0.02	0.00	0	1.00
W2	8-4	50x150	SI-G	0.01	0.00	0.01	0.00	0	1.00
W1	8-5	50x150	SI-G	0.02	0.00	0.02	0.00	0	1.00

Jnt Id.	Lumber	P/AF	M/ZF	CSI	SSI	Purlin Space	Ke
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LOADCASE 20: Lumber DOL = 0.00 1.2ПН+1.4МВ

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Truss Spacing: 900

Loading (kN/m) (Global Reference Frame)

Panel		Start		End		
		Vert./ Normal	Horiz./ Parallel	Vert./ Normal	Horiz./ Parallel	
Верх. пояс						
T1	1-2	0.494	0.000	0.494	0.000	proj.
T1	2-10	0.497	0.000	0.497	0.000	proj.
T1	10-11	0.497	0.000	0.497	0.000	proj.
T1	11-3	0.497	0.000	0.497	0.000	proj.
T1	3-12	0.496	0.000	0.496	0.000	proj.
T1	12-4	0.496	0.000	0.496	0.000	proj.
T1	4-13	0.496	0.000	0.496	0.000	proj.
T1	13-5	0.496	0.000	0.496	0.000	proj.
T1	5-14	0.497	0.000	0.497	0.000	proj.
T1	14-15	0.497	0.000	0.497	0.000	proj.
T1	15-6	0.497	0.000	0.497	0.000	proj.
T1	6-7	0.494	0.000	0.494	0.000	proj.
Нижний пояс						
B1	2-8	0.352	0.000	0.352	0.000	proj.
B1	8-9	0.352	0.000	0.352	0.000	proj.
B2	9-6	0.352	0.000	0.352	0.000	proj.

Диагонали

Note: Loads not marked projected, (proj), are loads normal to and parallel to the member.

Member Forces (Local Reference Frame)

		Panel Start			Panel End			Max.
		Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Panel Moment (kNm)
Верх. пояс								
T1	1-2	0.000	0.000	0.000	0.274	0.113	-0.089	0.000
T1	2-3	0.410	-6.667	0.089	0.502	-6.277	-0.188	-0.137
T1	3-4	0.582	-5.182	0.188	0.424	-4.755	0.000	0.214
T1	4-5	0.424	-4.755	0.000	0.582	-5.182	-0.188	0.214
T1	5-6	0.502	-6.277	0.188	0.410	-6.667	-0.089	-0.137
T1	6-7	0.274	0.113	0.089	0.000	0.000	0.000	-0.022
Нижний пояс								
B1	2-8	0.565	5.968	0.000	0.915	5.968	-0.735	0.453
B1	8-9	0.915	5.968	0.735	-0.457	5.968	0.156	-0.215
B2	9-6	0.457	5.968	-0.156	0.565	5.968	0.000	0.453
Диагонали								
W1	3-8	0.000	-1.529	0.000	0.000	-1.529	0.000	0.000
W2	8-4	0.000	2.935	0.000	0.000	2.935	0.000	0.000
W1	8-5	0.000	-1.529	0.000	0.000	-1.529	0.000	0.000

Reactions (Global Reference Frame)

Joint Id.	Vertical (kN)	Horiz. (kN)	Moment (kNm)	Bearing Width (mm)
2	-3.860	0.000	0.000	
6	-3.860	0.000	0.000	

Stress Report for Loadcase 20.

Member Design Forces (0 Axial & Moment: NO stress check needed)

Panel		Start		Midpanel		End	
		Axial	Moment	Axial	Moment	Axial	Moment
Верх. пояс							
T1	1-2	0.000	0.000	0.000	0.000	0.113	-0.089
T1	2-3	-6.667	-0.091	-6.491	-0.137	-6.393	-0.188
T1	3-4	-5.182	-0.188	-5.162	0.214	-5.142	-0.070
T1	4-5	-5.142	-0.070	-5.162	0.214	-5.182	-0.188
T1	5-6	-6.393	-0.188	-6.491	-0.137	-6.667	-0.091
T1	6-7	0.113	-0.089	0.057	-0.022	0.000	0.000
Нижний пояс							
B1	2-8	5.968	0.000	5.968	0.453	5.968	-0.735
B1	8-9	5.968	-0.735	5.968	-0.215	5.968	0.156
B2	9-6	5.968	0.156	5.968	0.453	5.968	0.000
Диагонали							
W1	3-8	-1.529	0.000	-1.529	0.000	-1.529	0.000
W2	8-4	2.935	0.000	2.935	0.000	2.935	0.000
W1	8-5	-1.529	0.000	-1.529	0.000	-1.529	0.000

Stress Report for Loadcase 20.

Panel	Lumber			P/AF	M/ZF	CSI	SSI	Purlin Space	Ke
Верх. пояс									
T1	1-2	50x150	SI-G	0.00	0.05	0.05	0.04	0	2.00
T1	2-3	50x150	SI-G	0.10	0.11	0.21	0.07	0	0.39
T1	3-4	50x150	SI-G	0.09	0.13	0.22	0.08	0	0.60
T1	4-5	50x150	SI-G	0.09	0.13	0.22	0.08	0	0.35
T1	5-6	50x150	SI-G	0.10	0.11	0.21	0.07	0	0.41
T1	6-7	50x150	SI-G	0.00	0.05	0.05	0.04	0	2.00
Нижний пояс									
B1	2-8	50x150	SI-G	0.15	0.43	0.58	0.12	0	0.47
B1	8-9	50x150	SI-G	0.15	0.43	0.58	0.12	0	0.60
B2	9-6	50x150	SI-G	0.15	0.27	0.41	0.08	0	0.60
Диагонали									
W1	3-8	50x150	SI-G	0.03	0.00	0.03	0.00	0	1.00
W2	8-4	50x150	SI-G	0.07	0.00	0.07	0.00	0	1.00
W1	8-5	50x150	SI-G	0.03	0.00	0.03	0.00	0	1.00

Jnt Id.	Lumber	P/AF	M/ZF	CSI	SSI	Purlin Space	Ke
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LOADCASE 21: Lumber DOL = 3.00 Def: ПН+ВН+СНμ1/μ2

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Truss Spacing: 900

Loading (kN/m) (Global Reference Frame)

Panel		Start		End		
		Vert./ Normal	Horiz./ Parallel	Vert./ Normal	Horiz./ Parallel	
Верх. пояс						
T1	1-2	0.412	0.000	0.412	0.000	proj.
T1	2-10	0.414	0.000	0.414	0.000	proj.
T1	10-11	0.414	0.000	0.414	0.000	proj.
T1	11-3	0.414	0.000	0.414	0.000	proj.
T1	3-12	0.413	0.000	0.413	0.000	proj.
T1	12-4	0.413	0.000	0.413	0.000	proj.
T1	4-13	0.413	0.000	0.413	0.000	proj.
T1	13-5	0.413	0.000	0.413	0.000	proj.
T1	5-14	0.414	0.000	0.414	0.000	proj.
T1	14-15	0.414	0.000	0.414	0.000	proj.
T1	15-6	0.414	0.000	0.414	0.000	proj.
T1	6-7	0.412	0.000	0.412	0.000	proj.
Нижний пояс						
B1	2-8	0.293	0.000	0.293	0.000	proj.
B1	8-9	0.293	0.000	0.293	0.000	proj.
B2	9-6	0.293	0.000	0.293	0.000	proj.

Диагонали
Note: Loads not marked projected, (proj), are loads normal to and parallel to the member.

Member Forces (Local Reference Frame)

		Panel Start			Panel End			Max.
		Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Panel Moment (kNm)
Верх. пояс								
T1	1-2	0.000	0.000	0.000	0.228	0.094	-0.074	0.000
T1	2-3	0.342	-5.556	0.074	0.418	-5.231	-0.157	-0.114
T1	3-4	0.485	-4.318	0.157	0.353	-3.962	0.000	0.178
T1	4-5	0.353	-3.962	0.000	0.485	-4.318	-0.157	0.178
T1	5-6	0.418	-5.231	0.157	0.342	-5.556	-0.074	-0.114
T1	6-7	0.228	0.094	0.074	0.000	0.000	0.000	-0.018
Нижний пояс								
B1	2-8	0.470	4.974	0.000	0.762	4.974	-0.613	0.377
B1	8-9	0.762	4.974	0.613	-0.381	4.974	0.130	-0.179
B2	9-6	0.381	4.974	-0.130	0.470	4.974	0.000	0.377
Диагонали								
W1	3-8	0.000	-1.274	0.000	0.000	-1.274	0.000	0.000
W2	8-4	0.000	2.446	0.000	0.000	2.446	0.000	0.000
W1	8-5	0.000	-1.274	0.000	0.000	-1.274	0.000	0.000

Reactions (Global Reference Frame)

Joint Id.	Vertical (kN)	Horiz. (kN)	Moment (kNm)	Bearing Width (mm)
2	-3.216	0.000	0.000	
6	-3.216	0.000	0.000	

Deflection Report for Load Case 21 - (mm).

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Jnt Id.	X (Global Reference Frame)	Y	Z
1	-1	2	0
2	0	0	-0
3	2	-6	-0
4	1	-7	0
5	1	-6	0
6	3	0	0
2	0	0	-0
8	1	-7	0
3	2	-6	-0
8	1	-7	0
8	1	-7	0

Member	Lumber	Local calc. *** ALL	Local allow. LIVE	Global calc. LOADS	Global allow. ***	Start X (Local Reference Frame)	Joint Y	End X	Joint Y
T1 1-2	50x150 SI-G	0	6	2	600	-0	3	0	0
T1 2-3	50x150 SI-G	-1	22	-6	42	0	0	-1	-6
T1 3-4	50x150 SI-G	-4	24	-10	42	-1	-6	-1	-7
T1 4-5	50x150 SI-G	-4	24	-10	42	4	-6	3	-6
T1 5-6	50x150 SI-G	-1	22	-5	42	3	-5	3	1
T1 6-7	50x150 SI-G	0	6	2	600	3	1	3	4
B1 2-8	50x150 SI-G	-8	-1	-11	42	0	0	1	-7
B1 8-9	50x150 SI-G	-8	-1	-11	42	1	-7	3	0
B2 9-6	50x150 SI-G	-8	-1	-11	42	1	-7	3	0
W1 3-8	50x150 SI-G	0	-1	-6	-1	4	-5	4	-6
W2 8-4	50x150 SI-G	-0	-1	-1	-1	-7	-1	-7	-1
W1 8-5	50x150 SI-G	0	-1	-7	-1	-1	-7	-1	-6

LOADCASE 22: Lumber DOL = 3.00 Def: ПН+ВН+СН μ 2/ μ 1

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Truss Spacing: 900

Loading (kN/m) (Global Reference Frame)

Panel		Start		End		
		Vert./ Normal	Horiz./ Parallel	Vert./ Normal	Horiz./ Parallel	
Верх. пояс						
T1	1-2	0.412	0.000	0.412	0.000	proj.
T1	2-10	0.414	0.000	0.414	0.000	proj.
T1	10-11	0.414	0.000	0.414	0.000	proj.
T1	11-3	0.414	0.000	0.414	0.000	proj.
T1	3-12	0.413	0.000	0.413	0.000	proj.
T1	12-4	0.413	0.000	0.413	0.000	proj.
T1	4-13	0.413	0.000	0.413	0.000	proj.
T1	13-5	0.413	0.000	0.413	0.000	proj.
T1	5-14	0.414	0.000	0.414	0.000	proj.
T1	14-15	0.414	0.000	0.414	0.000	proj.
T1	15-6	0.414	0.000	0.414	0.000	proj.
T1	6-7	0.412	0.000	0.412	0.000	proj.
Нижний пояс						
B1	2-8	0.293	0.000	0.293	0.000	proj.
B1	8-9	0.293	0.000	0.293	0.000	proj.
B2	9-6	0.293	0.000	0.293	0.000	proj.

Диагонали

Note: Loads not marked projected, (proj), are loads normal to and parallel to the member.

Member Forces (Local Reference Frame)

		Panel Start			Panel End			Max.
		Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Panel Moment (kNm)
Верх. пояс								
T1	1-2	0.000	0.000	0.000	0.228	0.094	-0.074	0.000
T1	2-3	0.342	-5.556	0.074	0.418	-5.231	-0.157	-0.114
T1	3-4	0.485	-4.318	0.157	0.353	-3.962	0.000	0.178
T1	4-5	0.353	-3.962	0.000	0.485	-4.318	-0.157	0.178
T1	5-6	0.418	-5.231	0.157	0.342	-5.556	-0.074	-0.114
T1	6-7	0.228	0.094	0.074	0.000	0.000	0.000	-0.018
Нижний пояс								
B1	2-8	0.470	4.974	0.000	0.762	4.974	-0.613	0.377
B1	8-9	0.762	4.974	0.613	-0.381	4.974	0.130	-0.179
B2	9-6	0.381	4.974	-0.130	0.470	4.974	0.000	0.377
Диагонали								
W1	3-8	0.000	-1.274	0.000	0.000	-1.274	0.000	0.000
W2	8-4	0.000	2.446	0.000	0.000	2.446	0.000	0.000
W1	8-5	0.000	-1.274	0.000	0.000	-1.274	0.000	0.000

Reactions (Global Reference Frame)

Joint Id.	Vertical (kN)	Horiz. (kN)	Moment (kNm)	Bearing Width (mm)
2	-3.216	0.000	0.000	
6	-3.216	0.000	0.000	

Deflection Report for Load Case 22 - (mm).

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Jnt Id.	X (Global Reference Frame)	Y	Z
1	-1	2	0
2	0	0	-0
3	2	-6	-0
4	1	-7	0
5	1	-6	0
6	3	0	0
2	0	0	-0
8	1	-7	0
3	2	-6	-0
8	1	-7	0
8	1	-7	0

Member	Lumber	Local calc. *** ALL	Local allow. LIVE	Global calc. LOADS	Global allow. ***	Start X (Local Reference Frame)	Joint Y	End X	Joint Y
T1 1-2	50x150 SI-G	0	6	2	600	-0	3	0	0
T1 2-3	50x150 SI-G	-1	22	-7	42	0	0	-1	-7
T1 3-4	50x150 SI-G	-4	24	-11	42	-1	-7	-1	-7
T1 4-5	50x150 SI-G	-4	24	-9	42	4	-6	3	-5
T1 5-6	50x150 SI-G	-1	22	-5	42	3	-5	3	1
T1 6-7	50x150 SI-G	0	6	2	600	3	1	3	4
B1 2-8	50x150 SI-G	-8	-1	-11	42	0	0	1	-7
B1 8-9	50x150 SI-G	-8	-1	-11	42	1	-7	3	0
B2 9-6	50x150 SI-G	-8	-1	-11	42	1	-7	3	0
W1 3-8	50x150 SI-G	0	-1	-6	-1	4	-5	4	-6
W2 8-4	50x150 SI-G	-0	-1	-1	-1	-7	-1	-7	-1
W1 8-5	50x150 SI-G	0	-1	-7	-1	-1	-7	-1	-6

LOADCASE 23: Lumber DOL = 3.00 Def: ПН+ВН+СН μ_1/μ_2 +Вспр

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Truss Spacing: 900

Loading (kN/m) (Global Reference Frame)

Panel		Start		End		
		Vert./ Normal	Horiz./ Parallel	Vert./ Normal	Horiz./ Parallel	
Верх. пояс						
T1	1-2	0.412	0.000	0.412	0.000	proj.
T1	2-10	0.414	0.000	0.414	0.000	proj.
T1	10-11	0.414	0.000	0.414	0.000	proj.
T1	11-3	0.414	0.000	0.414	0.000	proj.
T1	3-12	0.413	0.000	0.413	0.000	proj.
T1	12-4	0.413	0.000	0.413	0.000	proj.
T1	4-13	0.413	0.000	0.413	0.000	proj.
T1	13-5	0.413	0.000	0.413	0.000	proj.
T1	5-14	0.414	0.000	0.414	0.000	proj.
T1	14-15	0.414	0.000	0.414	0.000	proj.
T1	15-6	0.414	0.000	0.414	0.000	proj.
T1	6-7	0.412	0.000	0.412	0.000	proj.
Нижний пояс						
B1	2-8	0.293	0.000	0.293	0.000	proj.
B1	8-9	0.293	0.000	0.293	0.000	proj.
B2	9-6	0.293	0.000	0.293	0.000	proj.

Диагонали

Note: Loads not marked projected, (proj), are loads normal to and parallel to the member.

Member Forces (Local Reference Frame)

		Panel Start			Panel End			Max.
		Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Panel Moment (kNm)
Верх. пояс								
T1	1-2	0.000	0.000	0.000	0.228	0.094	-0.074	0.000
T1	2-3	0.342	-5.556	0.074	0.418	-5.231	-0.157	-0.114
T1	3-4	0.485	-4.318	0.157	0.353	-3.962	0.000	0.178
T1	4-5	0.353	-3.962	0.000	0.485	-4.318	-0.157	0.178
T1	5-6	0.418	-5.231	0.157	0.342	-5.556	-0.074	-0.114
T1	6-7	0.228	0.094	0.074	0.000	0.000	0.000	-0.018
Нижний пояс								
B1	2-8	0.470	4.974	0.000	0.762	4.974	-0.613	0.377
B1	8-9	0.762	4.974	0.613	-0.381	4.974	0.130	-0.179
B2	9-6	0.381	4.974	-0.130	0.470	4.974	0.000	0.377
Диагонали								
W1	3-8	0.000	-1.274	0.000	0.000	-1.274	0.000	0.000
W2	8-4	0.000	2.446	0.000	0.000	2.446	0.000	0.000
W1	8-5	0.000	-1.274	0.000	0.000	-1.274	0.000	0.000

Reactions (Global Reference Frame)

Joint Id.	Vertical (kN)	Horiz. (kN)	Moment (kNm)	Bearing Width (mm)
2	-3.216	0.000	0.000	
6	-3.216	0.000	0.000	

Deflection Report for Load Case 23 - (mm).

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Jnt Id.	X (Global Reference Frame)	Y	Z
1	-1	2	0
2	0	0	-0
3	2	-6	-0
4	1	-6	0
5	1	-6	0
6	3	0	0
2	0	0	-0
8	1	-7	0
3	2	-6	-0
8	1	-7	0
8	1	-7	0

Member	Lumber	Local calc. *** ALL	Local allow. LIVE	Global calc. LOADS	Global allow. ***	Start X (Local Reference Frame)	Joint Y	End X	Joint Y
T1 1-2	50x150 SI-G	0	6	2	600	-0	2	0	0
T1 2-3	50x150 SI-G	-1	22	-6	42	0	0	-1	-6
T1 3-4	50x150 SI-G	-3	24	-10	42	-1	-6	-1	-6
T1 4-5	50x150 SI-G	-5	24	-10	42	4	-5	3	-5
T1 5-6	50x150 SI-G	-2	22	-5	42	3	-5	2	1
T1 6-7	50x150 SI-G	0	6	3	600	2	1	2	4
B1 2-8	50x150 SI-G	-7	-1	-11	42	0	0	1	-7
B1 8-9	50x150 SI-G	-7	-1	-11	42	1	-7	3	0
B2 9-6	50x150 SI-G	-7	-1	-11	42	1	-7	3	0
W1 3-8	50x150 SI-G	0	-1	-6	-1	4	-5	4	-6
W2 8-4	50x150 SI-G	-0	-1	-1	-1	-7	-1	-6	-1
W1 8-5	50x150 SI-G	0	-1	-7	-1	-1	-7	-2	-6

LOADCASE 25: Lumber DOL = 3.00 Def: ПН+ВН+0.5μ1CH_П

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Truss Spacing: 900

Loading (kN/m) (Global Reference Frame)

		Start		End		
Panel		Vert./ Normal	Horiz./ Parallel	Vert./ Normal	Horiz./ Parallel	
Верх. пояс						
T1	1-2	0.412	0.000	0.412	0.000	proj.
T1	2-10	0.414	0.000	0.414	0.000	proj.
T1	10-11	0.414	0.000	0.414	0.000	proj.
T1	11-3	0.414	0.000	0.414	0.000	proj.
T1	3-12	0.413	0.000	0.413	0.000	proj.
T1	12-4	0.413	0.000	0.413	0.000	proj.
T1	4-13	0.413	0.000	0.413	0.000	proj.
T1	13-5	0.413	0.000	0.413	0.000	proj.
T1	5-14	0.414	0.000	0.414	0.000	proj.
T1	14-15	0.414	0.000	0.414	0.000	proj.
T1	15-6	0.414	0.000	0.414	0.000	proj.
T1	6-7	0.412	0.000	0.412	0.000	proj.
Нижний пояс						
B1	2-8	0.293	0.000	0.293	0.000	proj.
B1	8-9	0.293	0.000	0.293	0.000	proj.
B2	9-6	0.293	0.000	0.293	0.000	proj.

Диагонали

Note: Loads not marked projected, (proj), are loads normal to and parallel to the member.

Member Forces (Local Reference Frame)

		Panel Start			Panel End			Max.
		Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Panel Moment
Верх. пояс								
T1	1-2	0.000	0.000	0.000	0.228	0.094	-0.074	0.000
T1	2-3	0.342	-5.556	0.074	0.418	-5.231	-0.157	-0.114
T1	3-4	0.485	-4.318	0.157	0.353	-3.962	0.000	0.178
T1	4-5	0.353	-3.962	0.000	0.485	-4.318	-0.157	0.178
T1	5-6	0.418	-5.231	0.157	0.342	-5.556	-0.074	-0.114
T1	6-7	0.228	0.094	0.074	0.000	0.000	0.000	-0.018
Нижний пояс								
B1	2-8	0.470	4.974	0.000	0.762	4.974	-0.613	0.377
B1	8-9	0.762	4.974	0.613	-0.381	4.974	0.130	-0.179
B2	9-6	0.381	4.974	-0.130	0.470	4.974	0.000	0.377
Диагонали								
W1	3-8	0.000	-1.274	0.000	0.000	-1.274	0.000	0.000
W2	8-4	0.000	2.446	0.000	0.000	2.446	0.000	0.000
W1	8-5	0.000	-1.274	0.000	0.000	-1.274	0.000	0.000

Reactions (Global Reference Frame)

Joint Id.	Vertical (kN)	Horiz. (kN)	Moment (kNm)	Bearing Width (mm)
2	-3.216	0.000	0.000	
6	-3.216	0.000	0.000	

Deflection Report for Load Case 25 - (mm).

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Jnt Id.	X (Global Reference Frame)	Y	Z
1	-0	1	0
2	0	0	-0
3	1	-3	-0
4	1	-3	0
5	0	-3	0
6	1	0	0
2	0	0	-0
8	1	-4	0
3	1	-3	-0
8	1	-4	0
8	1	-4	0

Member	Lumber	Local calc. *** ALL	Local allow. LIVE	Global calc. LOADS	Global allow. ***	Start X (Local Reference Frame)	Joint Y	End X	Joint Y
T1 1-2	50x150 SI-G	0	6	1	600	-0	1	0	0
T1 2-3	50x150 SI-G	-0	22	-3	42	0	0	-0	-3
T1 3-4	50x150 SI-G	-1	24	-5	42	-0	-3	-1	-3
T1 4-5	50x150 SI-G	-3	24	-5	42	2	-3	2	-3
T1 5-6	50x150 SI-G	-1	22	-3	42	2	-3	1	1
T1 6-7	50x150 SI-G	0	6	1	600	1	1	1	2
B1 2-8	50x150 SI-G	-7	-1	-9	42	0	0	1	-4
B1 8-9	50x150 SI-G	-7	-1	-9	42	1	-4	1	0
B2 9-6	50x150 SI-G	-7	-1	-9	42	1	-4	1	0
W1 3-8	50x150 SI-G	0	-1	-3	-1	2	-2	2	-3
W2 8-4	50x150 SI-G	-0	-1	-1	-1	-4	-1	-3	-1
W1 8-5	50x150 SI-G	0	-1	-4	-1	-1	-4	-1	-3

LOADCASE 26: Lumber DOL = 3.00 Def: ПН+ВН+0.5μ1CH_Л

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Truss Spacing: 900

Loading (kN/m) (Global Reference Frame)

Panel	Start		End		
	Vert./ Normal	Horiz./ Parallel	Vert./ Normal	Horiz./ Parallel	
Верх. пояс					
T1 1-2	0.412	0.000	0.412	0.000	proj.
T1 2-10	0.414	0.000	0.414	0.000	proj.
T1 10-11	0.414	0.000	0.414	0.000	proj.
T1 11-3	0.414	0.000	0.414	0.000	proj.
T1 3-12	0.413	0.000	0.413	0.000	proj.
T1 12-4	0.413	0.000	0.413	0.000	proj.
T1 4-13	0.413	0.000	0.413	0.000	proj.
T1 13-5	0.413	0.000	0.413	0.000	proj.
T1 5-14	0.414	0.000	0.414	0.000	proj.
T1 14-15	0.414	0.000	0.414	0.000	proj.
T1 15-6	0.414	0.000	0.414	0.000	proj.
T1 6-7	0.412	0.000	0.412	0.000	proj.
Нижний пояс					
B1 2-8	0.293	0.000	0.293	0.000	proj.
B1 8-9	0.293	0.000	0.293	0.000	proj.
B2 9-6	0.293	0.000	0.293	0.000	proj.

Диагонали

Note: Loads not marked projected, (proj), are loads normal to and parallel to the member.

Member Forces (Local Reference Frame)

		Panel Start			Panel End			Max.
		Shear Force	Axial Force	Moment	Shear Force	Axial Force	Moment	Panel Moment
		(kN)	(kN)	(kNm)	(kN)	(kN)	(kNm)	(kNm)
Верх. пояс								
T1	1-2	0.000	0.000	0.000	0.228	0.094	-0.074	0.000
T1	2-3	0.342	-5.556	0.074	0.418	-5.231	-0.157	-0.114
T1	3-4	0.485	-4.318	0.157	0.353	-3.962	0.000	0.178
T1	4-5	0.353	-3.962	0.000	0.485	-4.318	-0.157	0.178
T1	5-6	0.418	-5.231	0.157	0.342	-5.556	-0.074	-0.114
T1	6-7	0.228	0.094	0.074	0.000	0.000	0.000	-0.018
Нижний пояс								
B1	2-8	0.470	4.974	0.000	0.762	4.974	-0.613	0.377
B1	8-9	0.762	4.974	0.613	-0.381	4.974	0.130	-0.179
B2	9-6	0.381	4.974	-0.130	0.470	4.974	0.000	0.377
Диагонали								
W1	3-8	0.000	-1.274	0.000	0.000	-1.274	0.000	0.000
W2	8-4	0.000	2.446	0.000	0.000	2.446	0.000	0.000
W1	8-5	0.000	-1.274	0.000	0.000	-1.274	0.000	0.000

Reactions (Global Reference Frame)

Joint Id.	Vertical (kN)	Horiz. (kN)	Moment (kNm)	Bearing Width (mm)
2	-3.216	0.000	0.000	
6	-3.216	0.000	0.000	

Deflection Report for Load Case 26 - (mm).

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Jnt Id.	X (Global Reference Frame)	Y	Z
1	-1	1	0
2	0	0	-0
3	1	-3	-0
4	1	-3	0
5	0	-3	0
6	1	0	0
2	0	0	-0
8	1	-4	0
3	1	-3	-0
8	1	-4	0
8	1	-4	0

Member	Lumber	Local calc. *** ALL	Local allow. LIVE	Global calc. LOADS	Global allow. ***	Start X (Local Reference Frame)	Joint Y	End X	Joint Y
T1 1-2	50x150 SI-G	0	6	1	600	-0	1	0	0
T1 2-3	50x150 SI-G	-1	22	-4	42	0	0	-0	-4
T1 3-4	50x150 SI-G	-3	24	-6	42	-0	-4	-1	-3
T1 4-5	50x150 SI-G	-1	24	-4	42	2	-3	2	-3
T1 5-6	50x150 SI-G	-0	22	-3	42	2	-3	1	1
T1 6-7	50x150 SI-G	0	6	1	600	1	1	1	2
B1 2-8	50x150 SI-G	-7	-1	-9	42	0	0	1	-4
B1 8-9	50x150 SI-G	-7	-1	-9	42	1	-4	1	0
B2 9-6	50x150 SI-G	-7	-1	-9	42	1	-4	1	0
W1 3-8	50x150 SI-G	0	-1	-3	-1	2	-3	2	-3
W2 8-4	50x150 SI-G	-0	-1	-1	-1	-4	-1	-3	-1
W1 8-5	50x150 SI-G	0	-1	-4	-1	-1	-4	-1	-3

LOADCASE 27: Lumber DOL = 3.00 Def: ПН+ВН+0.5μ1CH_П+Вспр

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Truss Spacing: 900

Loading (kN/m) (Global Reference Frame)

Panel		Start		End		
		Vert./ Normal	Horiz./ Parallel	Vert./ Normal	Horiz./ Parallel	
Верх. пояс						
T1	1-2	0.412	0.000	0.412	0.000	proj.
T1	2-10	0.414	0.000	0.414	0.000	proj.
T1	10-11	0.414	0.000	0.414	0.000	proj.
T1	11-3	0.414	0.000	0.414	0.000	proj.
T1	3-12	0.413	0.000	0.413	0.000	proj.
T1	12-4	0.413	0.000	0.413	0.000	proj.
T1	4-13	0.413	0.000	0.413	0.000	proj.
T1	13-5	0.413	0.000	0.413	0.000	proj.
T1	5-14	0.414	0.000	0.414	0.000	proj.
T1	14-15	0.414	0.000	0.414	0.000	proj.
T1	15-6	0.414	0.000	0.414	0.000	proj.
T1	6-7	0.412	0.000	0.412	0.000	proj.
Нижний пояс						
B1	2-8	0.293	0.000	0.293	0.000	proj.
B1	8-9	0.293	0.000	0.293	0.000	proj.
B2	9-6	0.293	0.000	0.293	0.000	proj.

Диагонали

Note: Loads not marked projected, (proj), are loads normal to and parallel to the member.

Member Forces (Local Reference Frame)

		Panel Start			Panel End			Max.
		Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Panel Moment (kNm)
Верх. пояс								
T1	1-2	0.000	0.000	0.000	0.228	0.094	-0.074	0.000
T1	2-3	0.342	-5.556	0.074	0.418	-5.231	-0.157	-0.114
T1	3-4	0.485	-4.318	0.157	0.353	-3.962	0.000	0.178
T1	4-5	0.353	-3.962	0.000	0.485	-4.318	-0.157	0.178
T1	5-6	0.418	-5.231	0.157	0.342	-5.556	-0.074	-0.114
T1	6-7	0.228	0.094	0.074	0.000	0.000	0.000	-0.018
Нижний пояс								
B1	2-8	0.470	4.974	0.000	0.762	4.974	-0.613	0.377
B1	8-9	0.762	4.974	0.613	-0.381	4.974	0.130	-0.179
B2	9-6	0.381	4.974	-0.130	0.470	4.974	0.000	0.377
Диагонали								
W1	3-8	0.000	-1.274	0.000	0.000	-1.274	0.000	0.000
W2	8-4	0.000	2.446	0.000	0.000	2.446	0.000	0.000
W1	8-5	0.000	-1.274	0.000	0.000	-1.274	0.000	0.000

Reactions (Global Reference Frame)

Joint Id.	Vertical (kN)	Horiz. (kN)	Moment (kNm)	Bearing Width (mm)
2	-3.216	0.000	0.000	
6	-3.216	0.000	0.000	

Deflection Report for Load Case 27 - (mm).

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Jnt Id.	X (Global Reference Frame)	Y	Z
1	-0	1	0
2	0	0	-0
3	1	-3	-0
4	1	-3	0
5	0	-3	0
6	1	0	0
2	0	0	-0
8	1	-3	0
3	1	-3	-0
8	1	-3	0
8	1	-3	0

Member	Lumber	Local calc. *** ALL	Local allow. LIVE	Global calc. LOADS	Global allow. ***	Start X (Local Reference Frame)	Joint Y	End X	Joint Y
T1 1-2	50x150 SI-G	0	6	1	600	-0	1	0	0
T1 2-3	50x150 SI-G	-0	22	-3	42	0	0	-0	-3
T1 3-4	50x150 SI-G	-1	24	-4	42	-0	-3	-1	-3
T1 4-5	50x150 SI-G	-3	24	-5	42	2	-3	2	-3
T1 5-6	50x150 SI-G	-1	22	-3	42	2	-3	1	1
T1 6-7	50x150 SI-G	0	6	2	600	1	1	1	2
B1 2-8	50x150 SI-G	-7	-1	-8	42	0	0	1	-3
B1 8-9	50x150 SI-G	-7	-1	-8	42	1	-3	1	0
B2 9-6	50x150 SI-G	-7	-1	-8	42	1	-3	1	0
W1 3-8	50x150 SI-G	0	-1	-3	-1	2	-2	2	-3
W2 8-4	50x150 SI-G	-0	-1	-1	-1	-3	-1	-3	-1
W1 8-5	50x150 SI-G	0	-1	-3	-1	-1	-3	-1	-3

LOADCASE 28: Lumber DOL = 3.00 Def: ПН+ВН+0.5μ1CH_Л +Всл

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Truss Spacing: 900

Loading (kN/m) (Global Reference Frame)

Panel	Start		End			
	Vert./ Normal	Horiz./ Parallel	Vert./ Normal	Horiz./ Parallel		
Верх. пояс						
T1	1-2	0.412	0.000	0.412	0.000	proj.
T1	2-10	0.414	0.000	0.414	0.000	proj.
T1	10-11	0.414	0.000	0.414	0.000	proj.
T1	11-3	0.414	0.000	0.414	0.000	proj.
T1	3-12	0.413	0.000	0.413	0.000	proj.
T1	12-4	0.413	0.000	0.413	0.000	proj.
T1	4-13	0.413	0.000	0.413	0.000	proj.
T1	13-5	0.413	0.000	0.413	0.000	proj.
T1	5-14	0.414	0.000	0.414	0.000	proj.
T1	14-15	0.414	0.000	0.414	0.000	proj.
T1	15-6	0.414	0.000	0.414	0.000	proj.
T1	6-7	0.412	0.000	0.412	0.000	proj.
Нижний пояс						
B1	2-8	0.293	0.000	0.293	0.000	proj.
B1	8-9	0.293	0.000	0.293	0.000	proj.
B2	9-6	0.293	0.000	0.293	0.000	proj.

Диагонали

Note: Loads not marked projected, (proj), are loads normal to and parallel to the member.

Member Forces (Local Reference Frame)

		Panel Start			Panel End			Max.
		Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Shear Force (kN)	Axial Force (kN)	Moment (kNm)	Panel Moment
Верх. пояс								
T1	1-2	0.000	0.000	0.000	0.228	0.094	-0.074	0.000
T1	2-3	0.342	-5.556	0.074	0.418	-5.231	-0.157	-0.114
T1	3-4	0.485	-4.318	0.157	0.353	-3.962	0.000	0.178
T1	4-5	0.353	-3.962	0.000	0.485	-4.318	-0.157	0.178
T1	5-6	0.418	-5.231	0.157	0.342	-5.556	-0.074	-0.114
T1	6-7	0.228	0.094	0.074	0.000	0.000	0.000	-0.018
Нижний пояс								
B1	2-8	0.470	4.974	0.000	0.762	4.974	-0.613	0.377
B1	8-9	0.762	4.974	0.613	-0.381	4.974	0.130	-0.179
B2	9-6	0.381	4.974	-0.130	0.470	4.974	0.000	0.377
Диагонали								
W1	3-8	0.000	-1.274	0.000	0.000	-1.274	0.000	0.000
W2	8-4	0.000	2.446	0.000	0.000	2.446	0.000	0.000
W1	8-5	0.000	-1.274	0.000	0.000	-1.274	0.000	0.000

Reactions (Global Reference Frame)

Joint Id.	Vertical (kN)	Horiz. (kN)	Moment (kNm)	Bearing Width (mm)
2	-3.216	0.000	0.000	
6	-3.216	0.000	0.000	

Deflection Report for Load Case 28 - (mm).

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Jnt Id.	X (Global Reference Frame)	Y	Z
1	-1	2	0
2	0	0	-0
3	1	-3	-0
4	1	-3	0
5	1	-3	0
6	1	0	0
2	0	0	-0
8	1	-3	0
3	1	-3	-0
8	1	-3	0
8	1	-3	0

Member	Lumber	Local calc. *** ALL	Local allow. LIVE	Global calc. LOADS	Global allow. ***	Start X (Local Reference Frame)	Joint Y	End X	Joint Y
T1 1-2	50x150 SI-G	0	6	2	600	-0	2	0	0
T1 2-3	50x150 SI-G	-1	22	-4	42	0	0	-0	-4
T1 3-4	50x150 SI-G	-3	24	-6	42	-0	-4	-1	-3
T1 4-5	50x150 SI-G	-1	24	-3	42	2	-3	2	-2
T1 5-6	50x150 SI-G	-0	22	-2	42	2	-2	1	1
T1 6-7	50x150 SI-G	0	6	1	600	1	1	1	2
B1 2-8	50x150 SI-G	-7	-1	-9	42	0	0	1	-3
B1 8-9	50x150 SI-G	-7	-1	-9	42	1	-3	1	0
B2 9-6	50x150 SI-G	-7	-1	-9	42	1	-3	1	0
W1 3-8	50x150 SI-G	0	-1	-3	-1	2	-3	2	-3
W2 8-4	50x150 SI-G	-0	-1	-1	-1	-3	-1	-3	-1
W1 8-5	50x150 SI-G	0	-1	-4	-1	-1	-4	-1	-3

DESIGN SUMMARY
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Stress Summary

Members	Size	Shear (kN)	Axial (kN)	PnlPnt Moment (kNm)	MidPnl Moment (kNm)	CSI	SSI	Allow. Purlin Spc (mm)
T1 1-2	50x150 SI-G	1.950(2)	0.403(2)	-0.633(2)	-0.158(2)	0.26(2)	0.17(2)	0
T1 2-3	50x150 SI-G	2.989(1)	-29.010(1)	-0.633(1)	0.804(1)	0.67(1)	0.27(1)	0
T1 3-4	50x150 SI-G	4.389(1)	-20.791(1)	-1.587(1)	-1.121(1)	0.85(1)	0.39(1)	0
T1 4-5	50x150 SI-G	3.704(1)	-19.391(1)	-0.698(1)	1.505(1)	0.83(1)	0.33(1)	0
T1 5-6	50x150 SI-G	3.865(1)	-29.251(1)	-1.591(1)	-1.194(1)	0.94(1)	0.35(1)	0
T1 6-7	50x150 SI-G	1.950(1)	0.403(1)	-0.633(1)	-0.158(1)	0.26(1)	0.17(1)	0
B1 2-8	50x150 SI-G	0.915(20)	28.836(1)	-0.735(20)	0.508(1)	0.71(4)	0.12(20)	0
B1 8-9	50x150 SI-G	0.915(20)	28.164(20)	-0.735(20)	-0.215(20)	0.70(3)	0.12(20)	0
B2 9-6	50x150 SI-G	0.598(1)	28.194(1)	0.253(1)	0.508(1)	0.66(1)	0.08(20)	0
W1 3-8	50x150 SI-G	0.000(1)	-11.724(1)	0.000(1)	0.000(1)	0.17(4)	0.00(1)	10000
W2 8-4	50x150 SI-G	0.000(1)	9.194(1)	0.000(1)	0.000(1)	0.15(1)	0.00(1)	10000
W1 8-5	50x150 SI-G	0.000(1)	-11.729(1)	0.000(1)	0.000(1)	0.17(3)	0.00(1)	10000

Maximum Reactions

Joint Id.	Vertical (kN)	Horiz. (kN)	Moment (kNm)	Bearing Width (mm)
2	-17.902(4)	0.759(6)	0.000(1)	125
6	-17.902(3)	0.000(4)	0.000(1)	125

Deflection Summary (mm)

Mbr	Local calc.	Local allow.	Global calc.	Global allow.	Start Joint ==X==	Joint ==Y==	End Joint ==X==	Joint ==Y==
T1 1-2	0(22)	6	2(22)	600	-3(1)	6(1)	0(1)	0(1)
T1 2-3	-1(22)	22	-7(22)	42	0(1)	0(1)	5(1)	-17(1)
T1 3-4	-4(22)	24	-11(22)	42	5(1)	-17(1)	4(1)	-18(1)
T1 4-5	-5(23)	24	-10(23)	42	4(1)	-18(1)	2(1)	-17(1)
T1 5-6	-2(23)	22	-5(21)	42	2(1)	-17(1)	7(1)	0(1)
T1 6-7	0(21)	6	3(23)	600	7(1)	0(1)	10(1)	7(1)
B1 2-8	-8(21)	-1	-11(21)	42	0(1)	0(1)	4(1)	-18(1)
B1 8-9	-8(22)	-1	-11(21)	42	4(1)	-18(1)	7(1)	0(1)
B2 9-6	-8(22)	-1	-11(21)	42	4(1)	-18(1)	7(1)	0(1)
W1 3-8	0(1)		-6(21)	-1	5(1)	-17(1)	4(1)	-18(1)
W2 8-4	-0(21)	-1	-1(22)	-1	4(1)	-18(1)	4(1)	-18(1)
W1 8-5	0(22)	-1	-7(22)	-1	4(1)	-18(1)	2(1)	-17(1)

Информация о пластинах

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Факторы нагрузки

Сост.нагр.	Продолжительность Фактор	Пролет Фактор	Прод./Пролет Фактор
1	0.62	1.00	0.62
2	0.62	1.00	0.62
3	0.62	1.00	0.62
4	0.62	1.00	0.62
5	0.62	1.00	0.62
6	0.62	1.00	0.62
7	0.62	1.00	0.62
8	0.62	1.00	0.62
9	0.62	1.00	0.62
10	0.62	1.00	0.62
11	0.62	1.00	0.62
12	0.62	1.00	0.62
13	1.00	1.00	1.00
14	0.62	1.00	0.62
15	0.62	1.00	0.62
16	0.62	1.00	0.62
17	0.62	1.00	0.62
18	0.62	1.00	0.62
19	0.62	1.00	0.62
20	0.62	1.00	0.62

1. Соединения нормально размещены между 10% а 25% длины элемента.

2. Допуск размещения SD = 5 (mm)

Опис узла	Пластина	Распол.(x1)	(y1)	(x2)	(y2) (mm)						
2	TMB1	GNA20	1330	320	9						
Элем.	Нагр. Сила	Угол	Нагр/Пласт	Нагр/Волокна	Grip	Прод/Пролет	Red.	Площадь	(mm2)		
	(kN)	(deg)	(deg)	(deg)	(MPa)	Фактор	Фактор	Треб.	Акт.	CSI	
T1	2 -33.035	31.3	31.3	8.3	2.344	0.62	1.00	11350	17555		
B1	2 33.057	31.5	31.5	31.5	1.940	0.62	1.00	13722	15383		
Элем.	Нагр. R-Сила	Сила/S-линия	перп.	Сдвиг	T-Ang	T-Вел	S-Ang	S-Вел.	Площадь	(mm)	Pl.
	(kN)	(deg)	Сила(kN)	(kN)	(N/mm)	(N/mm)	(N/mm)	(N/mm)	Треб.	Акт.	CSI
T1	1 -33.035	8.3	4.790	-32.686	67.0	127.9	23.0	97.85	217	325	0.67
B1	1 33.057	8.5	-4.870	32.696	67.0	127.9	23.0	97.85	203	325	0.62
Элем.	Нагр.Сила	Перп.	Сред. Элем D	Тяга. P	Мин. Bite	CSI	Нагр. 0.55le	Редукция. CSI			
	К волокнам(kN)	Bite (mm)	(mm)	Дл. (mm)	Сила (kN)	(kN)	(mm)	Площадь(mm2)			

Опис узла	Пластина	Распол.(x1)	(y1)	(x2)	(y2) (mm)						
3	TMW	GNA20	820	0	0						
Элем.	Нагр. Сила	Угол	Нагр/Пласт	Нагр/Волокна	Grip	Прод/Пролет	Red.	Площадь	(mm2)		
	(kN)	(deg)	(deg)	(deg)	(MPa)	Фактор	Фактор	Треб.	Акт.	CSI	
T1	3 11.778	21.5	1.4	44.5	1.578	0.62	1.00	6009	6410		
W1	3 -9.455	2.1	18.0	18.0	1.232	0.62	1.00	6177	6206		
Элем.	Нагр. R-Сила	Сила/S-линия	перп.	Сдвиг	T-Ang	T-Вел	S-Ang	S-Вел.	Площадь	(mm)	Pl.
	(kN)	(deg)	Сила(kN)	(kN)	(N/mm)	(N/mm)	(N/mm)	(N/mm)	Треб.	Акт.	CSI
T1	3 11.778	44.5	8.256	8.400	46.9	139.3	136.9	110.4	60	111	0.54
W1	3 -11.729	43.1	-8.016	-8.563	46.9	139.3	136.9	110.4	34	111	0.31
Элем.	Нагр.Сила	Перп.	Сред. Элем D	Тяга. P	Мин. Bite	CSI	Нагр. 0.55le	Редукция. CSI			
	К волокнам(kN)	Bite (mm)	(mm)	Дл. (mm)	Сила (kN)	(kN)	(mm)	Площадь(mm2)			

Информация о пластинах

Опис узла	Пластина		Распол.(x1)		(y1)	(x2)	(y2) (mm)				
4	CCW		GNA20	1025	0	0					
Элем.	Нагр.	Сила	Угол	Нагр/Пласт	Нагр/Волокна	Grip	Прод/Пролет	Red.	Площадь		(mm2)
		(kN)	(deg)	(deg)	(deg)	(MPa)	Фактор	Факторr	Треб.	Акт.	
T1	2	9.245	15.1	74.9	7.9	1.864	0.62	1.00	3993	4733	
T1	1	-9.245	15.1	74.9	7.9	1.864	0.62	1.00	3993	4733	
W2	2	-9.194	90.0	0.0	0.0	2.307	0.62	1.00	3209	10054	
Элем.	Нагр.	R-Сила	Сила/S-линия	перп.	Сдвиг	T-Ang	T-Вел	S-Ang	S-Вел.	Площадь	(mm) Pl.
		(kN)	(deg)	Сила(kN)	(kN)		(N/mm)		(N/mm)	Треб.	Акт. CSI
Элем.	Нагр.	Сила	Перп.	Сред.	Элем D	Тяга. P	Мин. Bite	CSI	Нагр.	0.55le	Редукция. CSI
	К волокнам(kN)		Bite (mm)	(mm)	Дл.	(mm)	Сила	(kN)		(mm)	Площадь(mm2)

Опис узла	Пластина		Распол.(x1)		(y1)	(x2)	(y2) (mm)				
5	TMW		GNA20	820	0	0					
Элем.	Нагр.	Сила	Угол	Нагр/Пласт	Нагр/Волокна	Grip	Прод/Пролет	Red.	Площадь		(mm2)
		(kN)	(deg)	(deg)	(deg)	(MPa)	Фактор	Факторr	Треб.	Акт.	
T1	3	-11.778	21.5	1.4	44.5	1.578	0.62	1.00	6009	6410	
W1	3	9.455	2.1	18.0	18.0	1.232	0.62	1.00	6177	6206	
Элем.	Нагр.	R-Сила	Сила/S-линия	перп.	Сдвиг	T-Ang	T-Вел	S-Ang	S-Вел.	Площадь	(mm) Pl.
		(kN)	(deg)	Сила(kN)	(kN)		(N/mm)		(N/mm)	Треб.	Акт. CSI
T1	3	-11.778	44.5	8.256	-8.400	46.9	139.3	136.9	110.4	60	111 0.54
W1	3	11.729	43.1	-8.016	8.563	46.9	139.3	136.9	110.4	34	111 0.31
Элем.	Нагр.	Сила	Перп.	Сред.	Элем D	Тяга. P	Мин. Bite	CSI	Нагр.	0.55le	Редукция. CSI
	К волокнам(kN)		Bite (mm)	(mm)	Дл.	(mm)	Сила	(kN)		(mm)	Площадь(mm2)

Опис узла	Пластина		Распол.(x1)		(y1)	(x2)	(y2) (mm)				
6	TMB1		GNA20	1330	320	9					
Элем.	Нагр.	Сила	Угол	Нагр/Пласт	Нагр/Волокна	Grip	Прод/Пролет	Red.	Площадь		(mm2)
		(kN)	(deg)	(deg)	(deg)	(MPa)	Фактор	Факторr	Треб.	Акт.	
T1	1	33.035	31.3	31.3	8.3	2.344	0.62	1.00	11350	17555	
B2	1	-33.057	31.5	31.5	31.5	1.940	0.62	1.00	13722	15383	
Элем.	Нагр.	R-Сила	Сила/S-линия	перп.	Сдвиг	T-Ang	T-Вел	S-Ang	S-Вел.	Площадь	(mm) Pl.
		(kN)	(deg)	Сила(kN)	(kN)		(N/mm)		(N/mm)	Треб.	Акт. CSI
T1	2	33.035	8.3	4.790	32.686	67.0	127.9	23.0	97.85	217	325 0.67
B2	1	-33.057	8.5	-4.870	-32.696	67.0	127.9	23.0	97.85	203	325 0.62
Элем.	Нагр.	Сила	Перп.	Сред.	Элем D	Тяга. P	Мин. Bite	CSI	Нагр.	0.55le	Редукция. CSI
	К волокнам(kN)		Bite (mm)	(mm)	Дл.	(mm)	Сила	(kN)		(mm)	Площадь(mm2)

Информация о пластинах

Опис узла	Пластина	Распол.(x1)	(y1)	(x2)	(y2) (mm)					
8	CMWWW	GNA20	1530	0	0					
Элем.	Нагр. Сила	Угол	Нагр/Пласт	Нагр/Волокна	Grip	Прод/Пролет	Red.	Площадь	(mm2)	
	(kN)	(deg)	(deg)	(deg)	(MPa)	Фактор	Факторr	Треб.	Акт.	
B1	10 -5.572	19.0	19.0	19.0	2.269	0.62	0.91	2175	20435	
W2	1 9.194	90.0	90.0	0.0	1.810	0.62	0.91	4499	8680	
W1	3 -11.197	10.4	10.4	9.7	2.541	0.62	0.91	3903	4228	
W1	3 11.197	10.4	10.4	9.7	2.541	0.62	0.91	3903	4228	
Элем.	Нагр. R-Сила	Сила/S-линия	перп.	Сдвиг	T-Ang	T-Вел	S-Ang	S-Вел.	Площадь	Pl.
	(kN)	(deg)	Сила(kN)	(kN)		(N/mm)		(N/mm)	Треб.	Акт. CSI
B1	10 5.572	19.0	1.815	5.268	90.0	135.0	0.0	63.57	63	305 0.21
W2,W1	3 -12.119	22.3	-4.932	-12.053	71.3	129.0	161.3	91.89	90	240 0.38
W2,W1	3 12.119	22.3	4.932	-12.053	71.3	129.0	161.3	91.89	90	240 0.38
Элем.	Нагр.Сила	Перп.	Сред. Элем D	Тяга. P	Мин. Bite	CSI	Нагр. 0.55le	Редукция. CSI		
	К волокнам(kN)	Bite (mm)	(mm)	Дл. (mm)	Сила (kN)	(kN)	(mm)	Площадь(mm2)		

Опис узла	Пластина	Распол.(x1)	(y1)	(x2)	(y2) (mm)					
9	CS	GNA20	1320	0	0					
Элем.	Нагр. Сила	Угол	Нагр/Пласт	Нагр/Волокна	Grip	Прод/Пролет	Red.	Площадь	(mm2)	
	(kN)	(deg)	(deg)	(deg)	(MPa)	Фактор	Факторr	Треб.	Акт.	
B1	4 -28.839	0.8	0.8	0.8	2.858	0.62	1.00	8125	11397	
B2	4 28.839	0.8	0.8	0.8	2.858	0.62	1.00	8125	11397	
Элем.	Нагр. R-Сила	Сила/S-линия	перп.	Сдвиг	T-Ang	T-Вел	S-Ang	S-Вел.	Площадь	Pl.
	(kN)	(deg)	Сила(kN)	(kN)		(N/mm)		(N/mm)	Треб.	Акт. CSI
B1	4 -28.839	89.2	28.836	-0.424	0.0	187.2	90.0	63.57	84	132 0.63
B2	4 28.839	89.2	28.836	0.424	0.0	187.2	90.0	63.57	109	132 0.83
Элем.	Нагр.Сила	Перп.	Сред. Элем D	Тяга. P	Мин. Bite	CSI	Нагр. 0.55le	Редукция. CSI		
	К волокнам(kN)	Bite (mm)	(mm)	Дл. (mm)	Сила (kN)	(kN)	(mm)	Площадь(mm2)		

Estimate for one 1 ply truss

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Lumber: Total for one truss

Верх. пояс	50x150 SI SM-JD	(	0.079cub. m)
Нижний пояс	50x150 SI SM-JD	(	0.063cub. m)
Диагонали	50x150 SI SM-JD	(	0.046cub. m)
Total Volume		(	0.188cub. m)

Plates: Total for one truss

820	4 (	62016mm2)
1530	2 (	93940mm2)
1025	2 (	51240mm2)
1320	2 (	53856mm2)
1330	4 (	161040mm2)
Total Plate Area	(	422092mm2)

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